

### I-210 Connected Corridors Pilot

## Analysis, Modeling and Simulation Review





### Outline

- Overview of I-210 Pilot ICM Project
- Response Planning Needs
- Modeling Approach
- Modeled Elements
  - Roadway elements
  - Traffic signals
  - Transit services
- Calibration Approach
- Building the Traffic Demand
  - Traffic State Estimation for Prediction
  - Ensuring Input Data Quality
  - Demand Modeling
  - Route Choice Modeling

- Modeling Response Plans
  - Response Planning Modeling
- Using the Model
  - Model Execution
  - Corridor Evaluation
- Research and Partnering













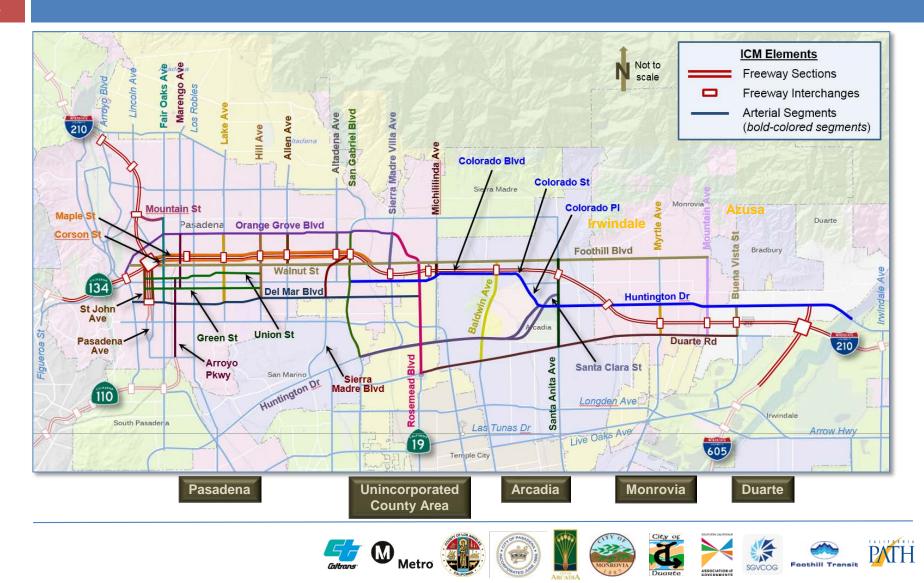


### I-210 Corridor Area

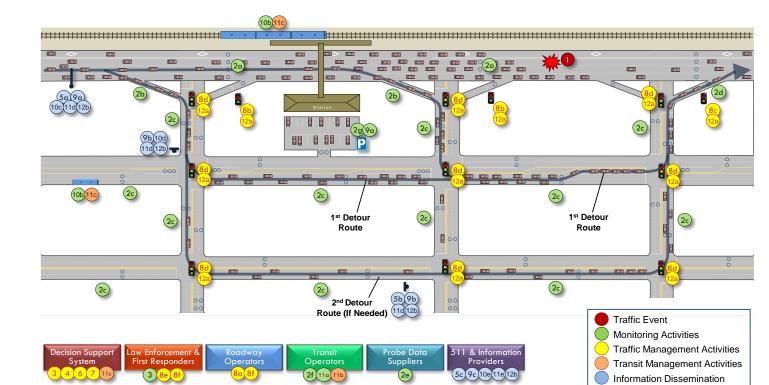




### I-210 Pilot ICM: Managed Roadways



### I-210 Pilot ICM: Responding to Incidents



- 1 Freeway Accident
- 2 Detection of Changing **Traffic Conditions**
- 3 Incident Identification
- 4 Incident Characterization
- 5 Incident Information Dissemination
- 6 Initial Impact Assessment
- 7 Response Planning
- 8 Implementation of Traffic Plan
- 9 Route Information Dissemination
- 10 Dissemination of Information about **Transit Options**
- 11 Transit Service Adjustments









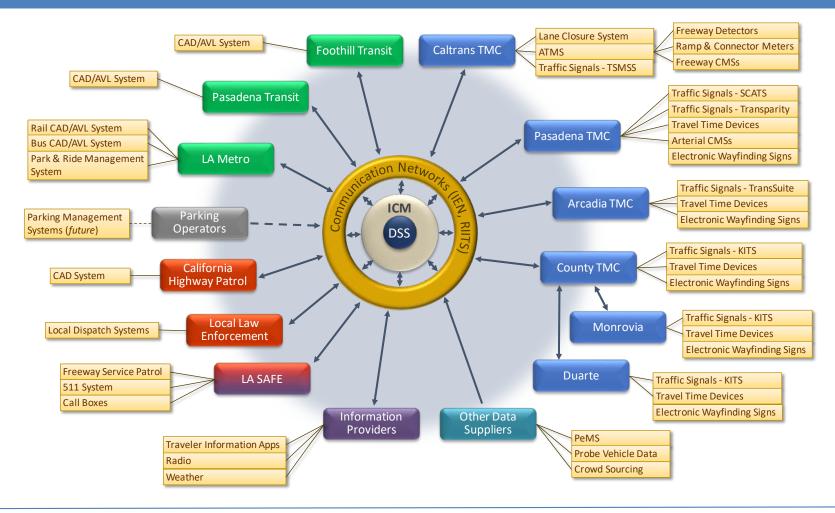








### I-210 Pilot ICM: Connected Systems



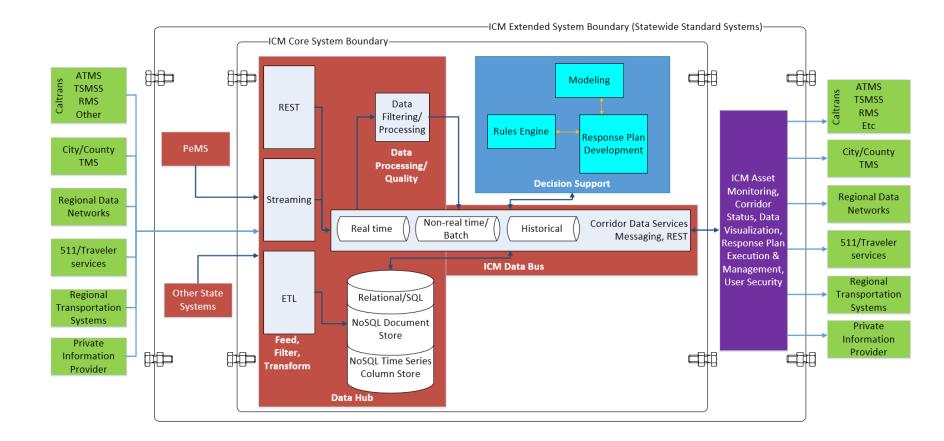




PATH

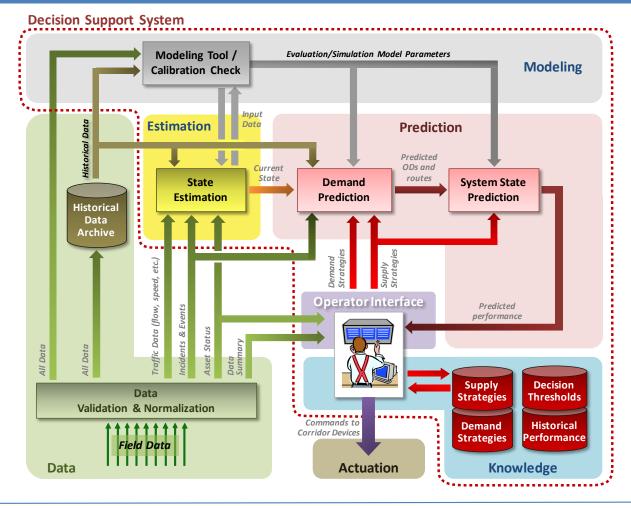
SGVCOG Foothill Transit

### **Proposed ICM Architecture**





### I-210 Pilot ICM: Decision Support System













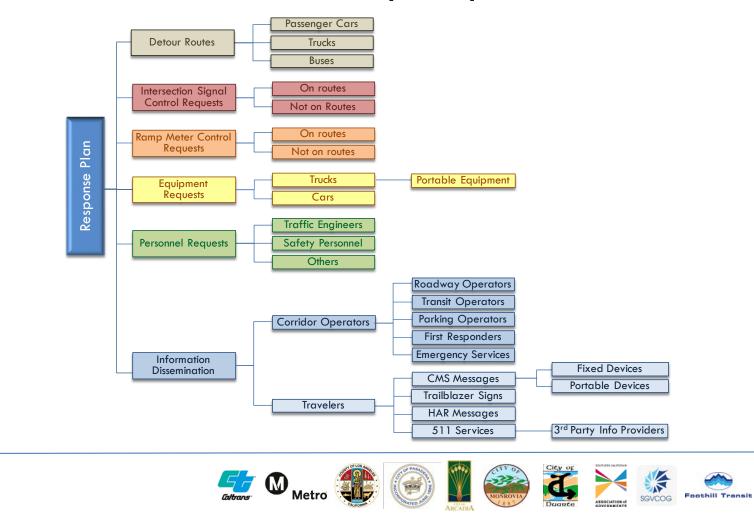




### **Response Plan Elements**

11

### Action items associated with a response plan



CALIFORNIA

### Preliminary Alternate Route "Menu"

~300 possible alternate arterial routes have been identified between Lake and Buena Vista interchanges within I-210 corridor



This set of 300 alternate routes is our "menu" of choices.

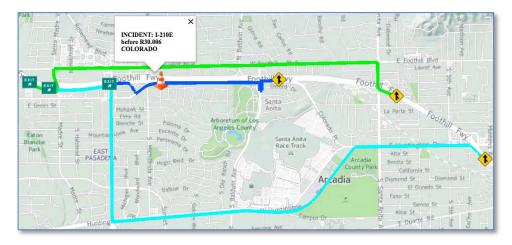


### Selection of Alternate Routes for Specific Incidents

Metro

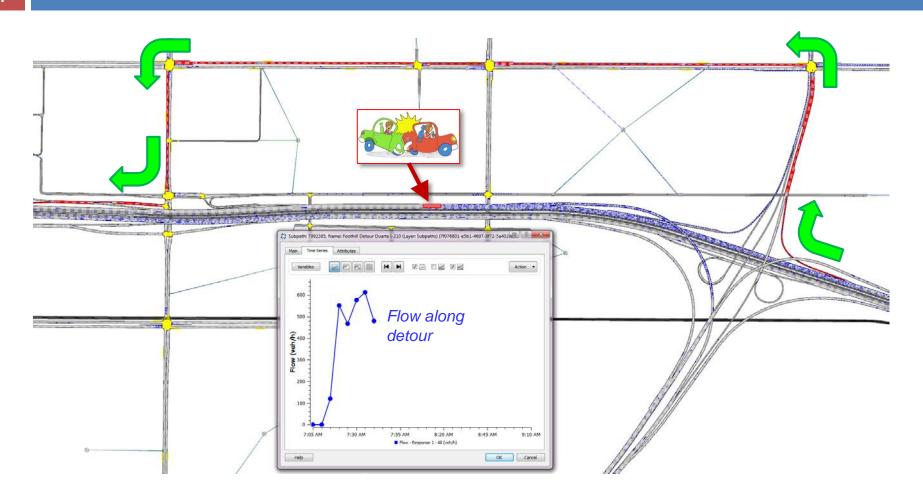
- Response to a given incident may include 1 to 3 alternate routes from the "menu" of ~300 preliminary routes
- Factors affecting choice
  - Location of incident
  - Prevailing traffic conditions on freeway and arterials
  - Ability of route to provide effective relief

Need to evaluate the impact of each response plan on corridor



Foothill Tr

### **Evaluating Potential Response Plans**





### Performance Metrics Produced by Aimsun

Measures Calculated by Aimsun	Value Standard Deviatio		Units
Network flow (throughput)	36,098.94	153.79	veh/h
Total Travelled Distance (VMT)	355,245.74	2,262.65	mi
Total Travel Time (VHT)	8,436.16	234.80	h
Travel time per mile	104.02	3.00	sec/mi
Delay per mile	41.03	3.00	sec/mi
Stop time per mile	22.12	2.13	sec/mi
Traffic density	14.51	0.42	veh/mi
Average network speed	41.25	0.73	mph
Total number of stops	36,663.48	1,607.83	#
Total number of lane changes	466,864.56	2,285.11	#
Number of lane changes per mile	1,599.70	7.83	#/mi
Fuel consumption	15,497.75	157.91	gal
Mean vehicle queue	480.65	34.46	veh

All metrics calculated for: - Each vehicle type

- All vehicles

Metrics also available for: - Specific subpaths



## **Target Performance Metrics**

#### Vehicle-based metrics

- Vehicle-miles traveled (VMT)
- Vehicle-hours of travel (VHT)
- Vehicle delays

#### Person-based metrics

- Person miles traveled (PMT)
- Person hours traveled (PHT)
- Delay

#### Travel times

- Average travel times / speeds
- Travel time reliability

#### Environmental metrics

- Fuel consumption
- Vehicle emissions

Requires assumption on average number of person per vehicle









# **Underlying Data Quality Needs**

Importance of high-quality data—including its timeliness, accuracy, and coverage—cannot be overstated

### Data quality affects

- Estimation of origin-destination trip patterns
- Network modeling and calibration
- Evaluation of response plans
- etc.

### Quality of work depends directly on quality of data

- $\blacksquare \text{ Missing data} \rightarrow \text{ reduced situational awareness}$ 
  - Unable to locate routes with available capacity
- Bad data → bad decisions
  - $\rightarrow$  Bad management and worse traffic
  - $\rightarrow$  Increased risk to pilot deployment





## **AMS Accomplishments**

#### Model development

- Completion of Synchro AM and PM models for control plan optimization
- Completion of I-210 Corridor Model Elements
  - ~1000 lane miles of road
  - ~5000 traffic detectors
  - 459 signalized intersections and control plans -- weekday/weekend
  - 45 freeway ramp meters -- TOD and LMR control plus queue / mainline override
  - Transit: Metro gold line and all bus routes
- Preliminary calibration of eastern subnetwork using 2008 SCAG data



## **AMS Accomplishments**

#### Corridor analyses

- Data/instrumentation gaps
- Operational needs
- Funding applications

#### Response planning

 Preliminary "menu" of alternate routes for response planning

#### Estimation

- Now running freeway estimation
- Feasible approach for arterial being tested

### Data quality

 Substantial data quality improvements in cooperation with stakeholders

#### Running the model

Aimsun running on the Amazon cloud



## AMS Schedule Moving Forward

21

	Milestones				
Date	2016	2016 2017			
	Q4	Q1	Q2	Q3	Q4
SCAG teamwork and OD data					
Obtain 2012 ODs over tier 3 TAZs					
New ODs imported to Aimsun					
Data quality and vetting data sources					
PeMS DQ report (weekly)					
Arterial DQ report (bi-weekly)					
Comparison of arterial mid-block, turning studies, and loop data					
Implemented method for loop data filtering / processing					
Arcadia loop data imported to Aimsun					
Pasadena loop data imported to Aimsun					
Calibration					
Calibration of eastern network portion using 2008 SCAG data					
Calibration of corridor freeway network using 2012 SCAG data					
Calibration of corridor freeway and arterial network					
Refinement of calibrated corridor network					
Build and evaluate response plans					
Reroute feasibility study					
Eastbound reroutes evaluation					
Westbound reroutes evaluation					
Reroutes evaluation refinement					
Management					
AMS Presentation					
FHWA AMS Workshop					















### **Purpose of Model**

Understand how to manage incidents on the I-210 corridor



- Take account of short-term traveler responses such as en route diversion triggered by
  - Unusual congestion
  - Information dissemination
- Guide the assembly and selection of appropriate response plans for potential deployment
- **Performance evaluation**













### Geographic Scope

#### Primary scope

Modeling of freeways and main arterials in Pasadena, Arcadia, Monrovia, and Duarte

#### Modeling extensions

- Modeling of freeway to Azusa to adequately capture bottlenecks
- Modeling of key arterials outside main area of interest to capture routing behavior that may effect corridor operations





## **Simulation Approaches**

### Possible modeling approaches

#### ■ Microscopic

- Modeling of individual vehicles
- Complex car-following and lane-changing models

#### Mesoscopic (micro-based)

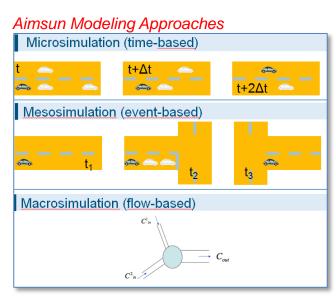
- Modeling of individual vehicles or groups of vehicles
- Simplified car-following model
- May or may not include lane modeling

#### Mesoscopic (macro-based)

- Behavior based on deterministic relationship between flow, speed, and density
- More detailed link-node representation than macro models (example: CTM model)

#### Macroscopic

- Behavior based on deterministic relationship between flow, speed, and density
- Simple link-node network representation



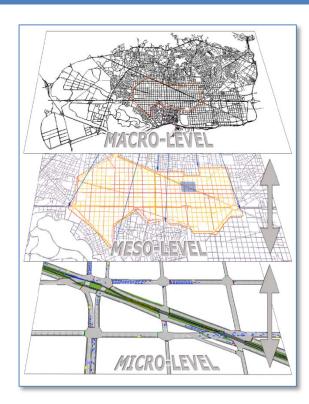
# Which Modeling Approach to Use?

### **Consideration** #1 – Simulation time

- Computational load increases with number of vehicles and network size, particularly with microscopic models
- Advantage to macroscopic models

#### **Consideration #2** – Replication of vehicle-based applications

- Microscopic approach allows a more accurate replication of applications based on individual vehicles
  - Vehicle-actuated traffic signal control
  - Vehicle-vehicle (V2V) and vehicleinfrastructure (V2I and I2V) applications













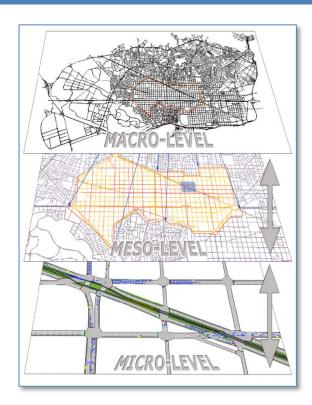
# Which Modeling Approach to Use?

### Consideration #3 – Calibration difficulty

- Calibration of large network a challenge regardless of the approach used
- Macroscopic approach generally simplest to calibrate
- Consideration #4 Ability to simulate new situations (prediction)
  - O-D based models better suited for evaluating:
    - New situations for which no data exists

Metro

Impacts of routing



## Selected Modeling Approach

### Hybrid simulation in Aimsun

- Microscopic simulation for mainline freeway and freeway ramps and some arterials
- Mesoscopic simulation for remainder of network

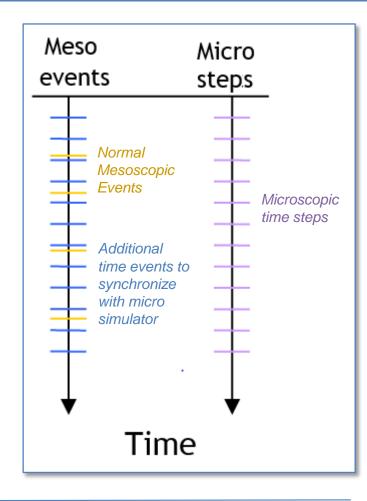




## Impacts on Modeling

- Different time processes in micro and meso model areas
  - Microscopic area: Time-based simulation
    - Simulation proceeds at fixed intervals
  - Mesoscopic area: Event-based simulation
    - Vehicle generation
    - Vehicle entrance in network
    - Vehicle node movement
    - Change in traffic signal state
    - Calculation of statistics
    - Change in traffic demand matrix

Metro



Foothill Tr

## Impacts on Modeling

### Different vehicle behavior models

- Microscopic model
  - Car-following and lane-changing model applied every time step
- Mesoscopic model
  - Vehicle only considered when entering and exiting a link → Movement within link not simulated
  - Calculates expected link exit time
  - Determines lane on which a vehicle would be at the end of a link

Metro

### Need to pay attention to traffic behavior at micro/meso boundary

Area where many previous models have failed





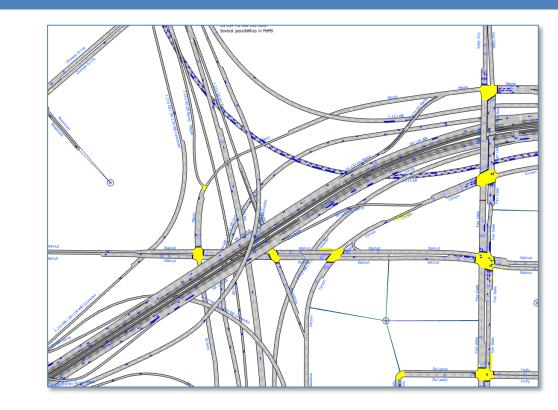
### **Roadway Segments**

#### **Roadway types**

- Freeways
- On/Off ramps
- **Arterials**
- Local streets

#### **Segment characteristics**

- Name
- Speed limits
- Lane width
- Lane restrictions
  - HOV
  - Truck







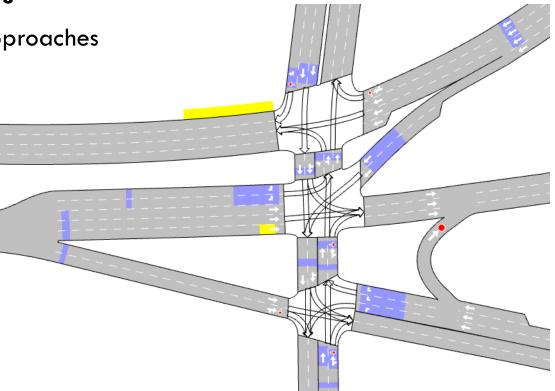




## Modeling of intersections

### Intersection movements

- Lane assignment on approaches
- Destination lanes
- Yielding movements
- Right-turn on red
- Turning bays
  - Length
- Traffic detectors
  - Size
  - Location







## Signal Timing Parameters

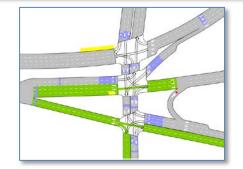
#### **Basic timing parameters**

- Cycle length
- Offset
- Phase sequence
- Phase durations

#### **Advanced features**

- Vehicle actuated control
  - Variable initial minimum green
  - Gradual reduction of allowed gap between vehicles to continue green
- **Detector operations** 
  - Detector calling/extending green
  - Type 3 detectors

rpe: Actuated ▼ Offset: 61.00 ♀ Yellow Time: 4.00 sec ♀ Cyde: 120 secs.
ngs: 2 🗧 Rest in Red 🗹 Single Entry Red Percentage: 50 🖨 Calculate Force-Offs
Timing Pre-emption
View as: Phases 🔹 🔍 🔍 Add Phase Delete Phase Delete All Phases
180         190         110         20         30         40         150         60         70           Barrier 1         Barrier 2         Barrier 3
Ring 1 115 55 325 55 145 55 195 55 195 55 1 2 3 4 5 6 7 8 9 10
Ring 2         30s         5s         13s         5s         43s         24s           11         12         13         14         15         16
Basics Actuated Detectors
Recall: Coord 🔻 🗹 Default Match Offset with: (1) End of Phase 🔿 Beginning of Phase
Minimum Green: 10.00 sec 🖨 Max-Out: 32.00 sec 🗣 Passage Time: 4.50 sec 🖨
Permissive Period Tro: 0.00 🖨 Force-Off: 0.00 sec
✓ Variable Initial         Maximum Initial Green:       25.00 sec         ♦         Hold
Gap Reduction Minimum Gap: 3.50 sec  Time Before Reduce: 15.00 sec  Time to Reduce: 15.00 sec















# Traffic Signals – Modeling Considerations

### Fixed-time control is no longer the default control mode

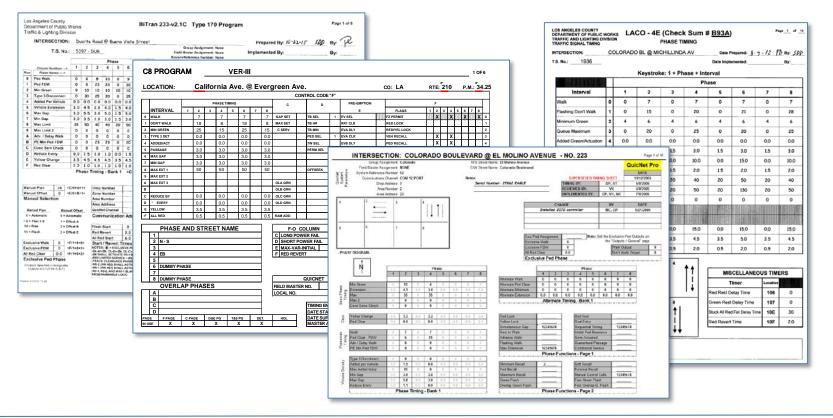
- 95+% of intersection in the I-210 corridor are actuated-coordinated
  - Fixed cycle length
  - Phase durations base on vehicle detection
  - Fixed offset point within cycle
- Real-time control at some intersections
  - SCATS
  - Systems allowing cycle and offset to be changed every 30 minutes
- Replication of specific control algorithms may require the development of Application Programming Interface modules







### Different control programs used by different agencies Need to develop a uniform modeling framework







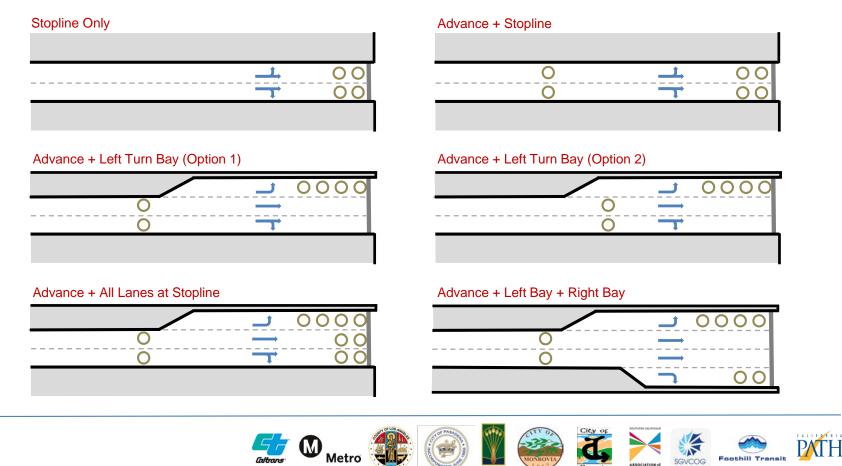








 Several types of detector configurations used within the corridor both across and within agencies



### How to organize timing data to facilitate maintenance

 Big issue if model is to be continuously used

### Scale of problem

- Many intersections use at least 3 different plans
- Newer 2070 controllers allow up to 64 plans
- Different timing schedules for week and weekend days
- Special control schedules for holidays

Event	Hour :	Min	Plan or Function	Sun 1	Mon 2	Tue 3	Wed 4	Thu 5	Fri 6	Sat 7
0	00:	00	E	х	x	х	x	х	х	X
1	06 :	00	2		X	х	х	х	х	
2	09:	00	1	х	X	х	x	х	х	X
3	15 :	30	3		X	х	х	х	х	
4	19 :	00	1		X	х	х	х	х	
5	21 :	00	E	х	X	×	х	х	X	X
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7	:									
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9	:									
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В	:									
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2	21 : 00	E		х	х	х	х	х	
3	:								
4	:								
5	:								
6	:								
7	:								
8	:								
9	:								
A	:								
в	:				_				
С	:								
D	:								
E	:								
F	:								

TABLE 1 - Time Of Day

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2	05 / 09	1		X					-				
3	09 / 01	1		X									
4	11 / 04	1					X						
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		7	12	/ 24	1			X	X	X	x	X	
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### **Example: Schedule of active timing plans from I-210 corridor**

In	tersection	8 8 8	8 5	8 8	8 8	9 9	8 5	8 8	9:30	11.30	14:00	14:45	15.00	15:45	16:15	16:30	18:00	19:00	19:15	0000	8 8	8 8	8 8	8	8	8 8	8 8	99	8 8	8	3:00	13:30	19:00	19:30	20:00	8	22:45
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Fair Oaks Ave	Peoria St	62 62 62		6 6	6 6	6 6	6 6	6 6	5 5	5 5		55	5 7	7	77	7 7	7	7 5					62 62		62				62 62	62 6	2 62		62 62		62 63		
Fair Oaks Ave	Villa St	62 62 62		6 6	6 6	6 6	6 6	6 6	5 5	5 5		5 5	5 7	7	77	7 7	7	77	7 5				62 62	62		62 5	5 5	5	5 5	5	5 5		5 5		62 63		62 62
Fair Oaks Ave	Maple St	EEE		1 1	1 1	1 1	1 1	1 1	5 2 5 2	2 2 2		33 33	3 3		3 4	4 4	4	4 5 4 5	5 5		EE		EE	E		E		5	5 5	~	55		5 5		EE		E E E E
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Fair Oaks Ave	Congress St	14 14 14		6 6	6 6	6 6	6 6	6 6	5 5	5 5		5 5	5 7	7	7 7	7 7	7	7 5	5 5		14 1		14 14	14		14 5	~ ~	5	55	5	5 5		5 5		5 5		5 14
Fair Oaks Ave	Fillmore St	14 14 14		6 6	6 6	6 6	6 6	6 6	5 5	5 5		5 5	5 7	7	7 7	7 7	7	7 5	5 5		14 1		14 14	14		14 5	5 5	5	5 5	5	5 5		5 5		14 14		14 14
Fair Oaks Ave	Bellefontaine St	14 14 14		6 6	6 6	6 6	6 6	6 6	5 5	5 5		55	5 7	7	7 7	7 7	7	7 5	5 5		14 1		14 14	14		14 5	5 5	5	5 5	5	5 5	5	5 5		14 14		14 14
Fair Oaks Ave	Glenarm St	14 14 14	6 6	6 6	6 6	6 6	6 6	6 6	5 5	5 5	5 !	55	5 7	7	7 7	7 7	7	7 5	5 5	5 14	14 1	4 14	14 14	14	14	14 5	5 5	5	5 5	5	55	5	5 5	5	14 14	1 14	14 14
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Arroyo Pkwy	Cordova St	EEE		1	1 1	1 1	1 1	1 1	5 2	2 2		3 3	3 3	3	3 4	4 4	4	4 4	4 5		EE		EE	E		E	5 5	5	5 5	5	5 5	5	5 5		EE		EE
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Arrovo Pkwy	Glenarm St	14 14 14		6 6	6 6	6 6	6 6	6 6	6 6	6 6	~ .	6 6	6 6	6	6 6	6 6	6	6 6	6 6		14 1		14 14	14		14 6	6	6	6 6	6	6 6	6	6 6		14 14		14 14
Raymond Ave	Del Mar Station	14 14 14		6 6	6 6	6 6	6 6	6 6	5 5	5 5		5 5	5 7	7	7 7	7 7	7	7 6	6 6		14 1		14 14	14	14	14 6	6 6	6	6 6	6	6 6	6	6 6	6	14 14		14 14
Raymond Ave	Pico Street	14 14 14	6 6	6 6	6 6	6 6	6 6	6 6	5 5	5 5	5 !	5 5	5 7	7	77	7 7	7	7 5	5 5	5 14	14 1	4 14	14 14	14	14	14 5	5 5	5	5 5	5	55	5	5 5	5	14 14	1 14	14 14
Raymond Ave	Fillmore St	14 14 14	6 6	6 6	6 6	6 6	6 6	6 6	5 5	5 5	5 !	55	5 7	7	77	7 7	7	7 5	5 5	5 14	14 1	4 14	14 14	14	14	14 5	5 5	5	5 5	5	55	5	5 5	5	14 14	1 14	14 14
Raymond Ave	Glenarm St	EEE	ΕE	ΕE	ΕE	ΕE	ΕE	ΕE	E E	ΕE	EI	ΕE	ΕE	ΕI	ΕE	E E	E	ΕE	ΕB	E E	E E	E E	EE	E	Е	ΕĒ	ΕE	E	E E	E	ΕE	E	E E	E	E E	E	ΕE
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Washington Blvd Washington Blvd	Raymond Ave	EEE				6 6	2 2	2 2	5 5	5 5	5		5 1			3 3		EE				EE	EE	E				-	EE	E	EE	E	EE	E	EE		EE
Washington Blvd	Marengo Ave	E 1 1	2 2		2 2	2 2	2 2	2 2	1 1	1 1	1	1 1	1 1			3 3		1 1	1 1	1 E		EE	EE			1	1 1			1					EE		EE
Washington Blvd	Los Robles Ave	EEE		6 6	6 6	6 6	6 6	6 6	5 5	5 5	5 !	55	5 7		7 7	7 7	7	7 5	5 5				EE	E		EE	ΕE	E	ΕE	E	E E	E	E E	E	EE	E	EE
Washington Blvd	El Molino Ave	E 1 1	2 2	2 2	2 2	2 2	2 2	2 2	1 1	1 1	1	1 1	1 1	1	3 3	3 3	1	1 1	1	1 1	1 .	1 1	1 E	E	Е	1	1 1	1	1 1	1	1 1	1	1 1	1	1 1	1	1 E
Washington Blvd	Lake Ave	62 62 62			6 6	6 6	6 6	6 6	5 5	5 5		55	5 7	7	77	7 7	7	7 5	5 5					E	E	E S	5 5	5	5 5	5	5 5	5	5 5	5	EE	E	ΕE
Washington Blvd	Catalina Ave	EEE		66	6 6	6 6	6 6	6 6	5 5	5 5		55	5 7		77	7 7	7	7 5			EE		EE	E	E	EE	ΕE	E	ΕE	E	ΕE	E	E E	E	E E	E	E E
Washington Blvd	Hill Ave	EEE		6 6	6 6	6 6	6 6	6 6	5 5	5 5	5 !	5 5	5 7	7	7 7	7 7	7	7 5	5 5	5 E	EE	EE	EE	E	E	EE	EE	E	E E	E	E E	E	E E	E	EE	E	EE
Washington Blvd	Sierra Bonita Ave	1 1 1 E 1 1	2 2		2 2	2 2	2 2	2 2	1 1	1 1	1	1 1	1 1	1	3 3	3 3	1	1 1	1	1 1	1 .		1 1 E E	1	F	1		1	1 1	1	1 1	1	1 1	1	F F	1	1 1
Washington Blvd Washington Blvd	Sinaloa Ave Allen Ave	EEE	2 2	2 2		6 6	6 6	2 2	5 5	5 5	5	1 1	5 5		3 3	3 3		1 1	5 5	1 E	EE		EE	E	E			5		5		-			C C	E	EE
Washington Blvd	Victory Park Entrance	EEE		E F	• •	E 1	1 2	2 2	2 2	2 2		2 2	1 1		2 2	2 2		2 5	5 5				EE	E	Ē	E		÷.	E E	E.	L L F F	F	EE	E E	EE	1	EE
Lake Ave	Claremont St	EEE		6 6	6 6	6 6	6 6	6 6	5 5	5 5		5 5	5 7	7	7 7	7 7	7	7 5	5 5				EE			E	5	5	5 5	5	5 5	5	5 5	5	E F	E	EE
Lake Ave	Belvidere St	EEE		6 6	6 6	6 6	6 6	6 6	5 5	5 5	5	5 5	5 7	7	7 7	7 7	7	7 5	5 5			ΕE	EE	E			5 5	5	5 5	5	5 5	5	5 5	5	EE		EE
Lake Ave	Macy's Crosswalk	EEE		66	6 6	6 6	6 6	6 6	52	2 2		22	2 2		2 3	3 3		3 3	5 5				EE	E		E S		5	5 5	•	5 5		5 5		EE		ΕE
Lake Ave	San Pasqual St	EEE		1 .	1 1	1 1	1 1	1 1	5 2	2 2		3 3	3 3		3 4	4 4		4 4	5 5			EE	EE	E		E	5 5	5	5 5		5 5	•	5 5		EE		ΕE
Hill Ave	San Pasqual St	FFF		6 6		6 6		6 6	5 5				5 7			7 7		7 5				5 5	5 F	F	F	F	5 5	5	55	5	5 5	5	5 5	5	5 5	E	EF
Allen Ave Mountain St / Sec	Casa Grande St Lincoln Ave	E E E 62 62 62		6 6	6 6	6 6	6 6	6 6	5 5	5 5	5 !	5 5	5 5	5	7 7	62 63	5	5 5	5 5	5 E	E E	E E	E E	E 62	E 62	E E	2 62	E 62 6	E 5	5	55 1262	5 62 I	55 6262	2 62	E E	E 2 62	E E 62 62
Mountain St / Sec	I-210 WB Off Ramp	62 62 62		2 2	2 2	2 2	2 2	1 1	1 1	1 1	1	1 1	1 2	2 3	2 2	2 2	2	2 2	2 1	2 14	14 1	4 14	14 14	52		14 1	4 14	14 -	14 14	14	02 102 14 14		62 62 14 14	1 14	14 14	2 62	62 62 14 14
Mountain St	PW-Yard Driveway Entra					2 1	1 1	1 1	1 1	1 1	1	1 1	1 2		2 2	2 2		2 62		2 62			62 62		62	62 6	2 62	62 F	52 62	62 6	2 62		62 62	2 62	62 6	2 62	62 62
Mountain St	Sunset Ave	EEE				2 2	2 2	2 2	5 5	5 5	5 !	5 5	5 3		3 3	3 3		E E			EE		EE			E	1 1	1	1 1	1	1 1	1	1 1	1	1 1	1	1 E
Mountain St	Fair Oaks Ave	EEE			6 6			6 6	5 5	5 5		5 5	5 7		7 7	7 7	7	7 5	5 5	5 E	EE	EE	EE	E	Е	EE	E	E	ΕĒ	E	ΕE	E	E E	E	E E	E	EE
Mountain St	Raymond Ave	1 1 1	2 2	2 2	2 2	2 2	2 2	2 2	1 1	1 1	1	1 1	1 1			3 3	1	1 1	1	1 1	1 *	1 1	1 1	1	1	1 1	1 1	1	1 1	1	1 1	1	1 1	1	1 1	1	1 1
Mountain St	Marengo Ave	1 1 1	2 2	2 2		2 2	22	2 2	1 1	1 1	1	1 1	1 1	1	3 3	3 3	1	1 1	1	1 1	1 *	1 1	1 1	1	1	1	1 1	1	1 1	1	1 1	1	1 1	1	1 1	1	1 1
Mountain St	Los Robles Ave	EEE				6 6	6 6	6 6	5 5	5 5		5 5	5 7	7	7 7	7 7	7	7 5	5 5		EE		EE	E	E	EE	E	E	ΕE	E	ΕE	E	E E	E	E E	E	EE
Mountain St Mountain St	El Molino Ave	1 1 1 E E E	2 2	2 2		2 Z	2 Z	2 2	3 3	3 3		33	3 1	1	1 1	1 1	1	1 1	1	1 1	1 1	1 1	1 1	1	1	1		1	1 1	1	1 1	1	1 1	1	1 1	1	
Mountain St Mountain St	Lake Ave Hill Ave	E E E 5 5 5		6 6		6 6	6 6	6 6	5 5	5 5		55	5 7	7	77	7 7	7	7 5 7 5	5 5		E E		E E 5 5	E		E	5 5	5	55	5	5 5	5	5 5	5	5 5	5	5 E
Mountain St	Allen Ave	5 5 5 E E E		6 6		6 6	6 6	6 6	5 5	5 5		55	5 5		7 7	7 7	5	5 5			D S		EE	E	F	E F	F	F	F F	F	FF	F	E E	F	E E	F	S S F F
Altadena Dr	Cooley Pl	EEE				5 5		EE	EE	EE		5 5	5 E		EE	EE		EE	EE		EE		EE	5	5	5 5	5 5	5	5 5	5	5 5	5	5 5	5	5 5	5	5 5
Altadena Dr	Paloma St	5 5 5	1 1	1 1	1 1	1 1	1 1	1 1	5 2	2 3		3 3	3 3		4 4	4 4		4 5	5 5		5 5		5 5		E	E	5 5	5	5 5	5	5 5	5	5 5	5	5 5		5 E
Sierra Madre Blvd	Sierra Madre Blvd	EEE							5 5				5 7	7	7 7	7 7	7	7 5	5 5	5 E	EE	EE	EE	E	Е	ΕE	ΕE	E	ΕE	E	ΕE	Е	E E	E	E E	E	E E
Sierra Madre Villa	Paloma St	EEE	6 6	66	66	6 6	6 6	6 6	5 5	5 5	5 !	55	5 7	7	7 7	7 7	7	7 5	5 5	5 E	EE	ΕE	EE	E	Е	E E	E	Е	ΕE	Е	ΕE	Е	E E	E	ΕE	E	ΕE



Metro











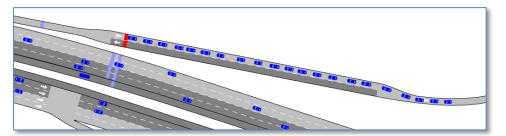


# 41 Ramp Metering Elements

## **Ramp Metering**

### Type of metering control

- Fixed
- Time-of-day
- Variable based on mainline vehicle detections
- Basic control parameters
  - Min/Max flow rate
  - Number of vehicles per green
  - Associated detectors



Name:	I-210 WB Altadena	External ID:
Type:	Flow	Platoons of: 1 vehicles/lane
3D Draw Mo	de: Traffic Light	•
Informatio	on	
Length: 6	.6 ft	
Distance f	from Entrance: 868.3 ft	
Distance t	to Exit: 0.0 ft	

Settings	
Control Type: External 🔻	
Flow (veh/h)	
Flow: 720.0000 veh/h	
Minimum Flow: 0.0000 veh/h 🚖 Maximum Flow: 1800.0000 veh/h 🖨	











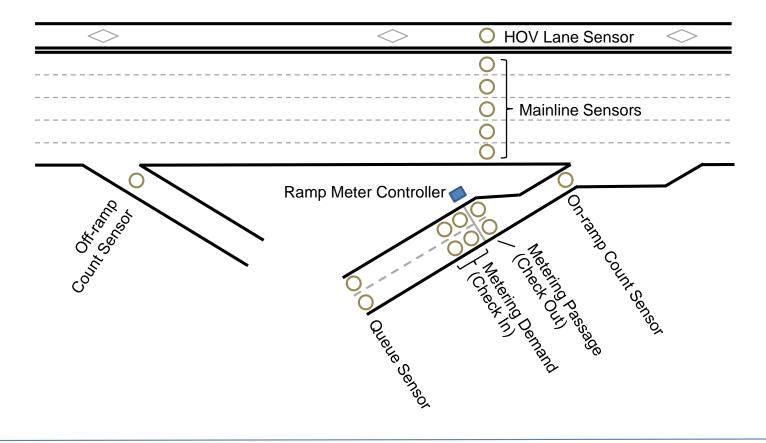




### **Ramp Metering Detectors**

43

### Typical detector configuration





## Ramp Metering – Modeling Considerations

- Ramp metering schedule
- Variations in metering operations across ramps

#### I-210 Ramp Metering Rate Schedule

		-								T																																					
Freeway	Ramp	Milepost	00:0	0:30	1:00	1:30	00.2	00.4	3-00	3:30	4:00	4:30	5:00	5:30	6:00	6:30	2:00	7:30	8:00	8:30	00:6	9:30	10:30	11:00	11:30	12:00	12:30	13:00	13:30	14:30	15:00	15:30	16:00	00:01	00:11	18-00	18-30	19-00	19-30	00.02	20:00	21:00	21-30	22:00	22:30	23:00	23:30
	Marengo	25.74																								12	12	12	12 1	2 12	12	12	12 :	12 1	.2 1	.2 1	2 1	2 1	2 1	2 1	.2						
	Lake	26.49																								12	12	12	12 1	2 12	12	12	12 :	12 1	.2 1	.2 1	2 1	2 1	2 1	2 1	.2						
	Hill	27.16																								10	10	10	10 1	0 10	10	10	10 :	10 1	.0 1	.0 1	0 1	0 1	0 1	0 1	.0						
	Allen	27.67																								10	10	10	10 1	0 10	10	10	10 :	10 1	.0 1	.0 1	0 1	0 1	0 1	0 1	.0						
	San Gabriel	28.70																								10	10	10	10 1	0 10	10	10	10 :	10 1	.0 1	.0 1	0 1	0 1	0 1	0 1	.0						
	Sierra Madre Villa	29.46																								10	10	10	10 1	0 10	10	10	10 :	10 1	.0 1	.0 1	0 1	0 1	01	01	.0						
	Rosemead	29.74																								10	10	10	10 1	0 10	10	10	10 :	10 1	.0 1	.0 1	0 1	0 1	0 1	0 1	.0						
	Michillinda	30.01																								10	10	10	10 1	0 10	10	10	10 :	10 1	.0 1	.0 1	0 1	0 1	0 1	0 1	.0						
	Baldwin	30.95																								10	10	10	10 1	0 10	10	10	10 :	10 1	.0 1	.0 1	0 1	0 1	0 1	0 1	.0						
	Santa Anita	32.06																								10	10	10	10 1	0 10	10	10	10	10 1	.0 1	.0 1	0 1	0 1	0 1	0 1	.0						
8	Huntington WB	32.86																								5	5	5	5	5 5	5	5	5	5	5	5	5	5	5	5	5						
I-210	Huntington EB	32.86																								10	10	10	10 1	0 10	10	10	10	10 1	.0 1	.0 1	0 1	0 1	0 1	0 1	.0						
-	Myrtle	34.15																								10	10	10	10 1	0 10	10	10	10 :	10 1	.0 1	.0 1	0 1	0 1	0 1	0 1	.0						
	Mountain	35.12																								10	10	10	10 1	0 10	10	10	10	L4 1	.4 1	.4 1	0 1	0 1	0 1	0 1	.0						
	Buena Vista	36.36																								10	10	10	10 1	0 10	10	10	10	14 1	.4 1	.4 1	0 1	0 1	0 1	0 1	.0						
	Mount Olive	36.33																								10	10	10	10 1	0 10	10	10	10 :	10 1	.0 1	.0 1	0 1	0 1	0 1	0 1	.0						
	I-605 Connector	36.60																								34	34	34	34 3	4 32	32	32	32	32 3	2 3	2 3	2 3	2 3	4 3	4 3	4						T
	Irwindale	37.92																								10	10	10	10 1	0 10	10	10	10 :	10 1	.0 1	.0 1	0 1	0 1	0 1	0 1	.0						T
	Vernon	39.05		Γ	Т	Т		Т							10	10	10	10	10	10 :	10 :	10 1	0 1	0 1	0 10	0 10	10	10	10 1	0 10	10	10	10 :	10 1	.0 1	.0 1	0 1	0 1	0 1	0 1	.0	T			T		Т
	Azusa SB	39.55			Τ	Τ		Τ							10	10	10	10	10	10 1	10 :	10 1	0 1	0 1	0 10	0 10	10	10	10 1	0 10	10	10	10	10 1	.0 1	.0 1	0 1	0 1	0 1	0 1	.0	T		T	T	T	Т
	Azusa NB	39.64			Т	Т		Т							10	10	10	10	10	10 :	10 :	10 1	0 1	0 1	0 10	0 10	10	10	10 1	0 10	10	10	10	10 1	.0 1	.0 1	0 1	0 1	0 1	0 1	.0	T			T	T	Т
	Citrus SB	40.56			Т	Т		Т							10	10	10	10	10	10 1	10 :	10 1	0 1	0 1	0 10	0 10	10	10	10 1	0 10	10	10	10	10 1	.0 1	.0 1	0 1	0 1	0 1	0 1	.0	T	T	T	T	T	Т
	Citrus NB	40.70		Т	Т	Т		Т	Т		Т				10	10	10	10	10	10 :	10 :	10 1	0 1	0 1	0 10	0 10	10	10	10 1	0 10	10	10	10	10 1	.0 1	.0 1	0 1	0 1	0 1	0 1	.0	T	T	T	T	T	Т
	Lake	26.14			1								14	14	14	14	14	14	14	14 :	14 :	14 1	.4 1	4 1	4 14	4 14	14	14	14 1	4 14	14	14	14 :	14 1	.4 1	.4 1	4 1	4 1	4 1	4 1	4						Т
	Hill	26.82			T	T		T					12	12	12	12	12	12	12	12 :	12 :	12 1	2 1	2 1	2 12	2 12	12	12	12 1	2 12	12	12	12	12 1	.2 1	.2 1	2 1	2 1	2 1	2 1	.2				T		Т
	Altadena	28.05			Т	Т		Т					10	10	10	10	10	12	12	10 :	10 :	10 1	0 1	0 1	0 10	0 10	10	10	10 1	0 10	10	10	10	10 1	.0 1	.0 1	0 1	0 1	0 1	0 1	.0	T	T	T	T	T	Т
	San Gabriel	28.29						T					10	10	10	10	10	12	12	10 :	10 :	10 1	0 1	0 1	0 10	0 10	10	10	10 1	0 10	10	10	10	10 1	.0 1	.0 1	0 1	0 1	0 1	0 1	.0	T			T		Т
	Sierra Madre Villa	29.19			T	T		T					5	5	5	5	9	9	9	9	6	6	6	6	66	56	6	6	6	6 6	6	6	6	6	6	6	6	6	6	6	6	T			T		T
	Rosemead SB	29.59			T	Т		T					10	10	10	10	10	10	10	10 :	10 :	10 1	0 1	0 1	0 10	0 10	10	10	10 1	0 10	10	10	10 :	10 1	.0 1	.0 1	0 1	0 1	0 1	0 1	0	T			T		Т
	Rosemead/Foothill	29.71			1	T		T					10	10	10	10	12	12	12	12 :	10 :	10 1	0 1	0 1	0 10	0 10	10	10	10 1	0 10	10	10	10	10 1	.0 1	.0 1	0 1	0 1	0 1	0 1	0						T
	Michillinda	29.85																				10 1																				T			T	-	Т
	Baldwin (North)	30.49		+	T	T		T			1		10	10	10	10	12	12	10	10 :	10 :	10 1	0 1	0 1	0 10	0 10	10	10	10 1	0 10	10	10	10	10 1	.0 1	.0 1	0 1	0 1	0 1	0 1	0	T	T		T	T	T
	Baldwin	30.49											10	10	10	10	10	10	10	10 :	10 :	10 1	0 1	0 1	0 10	0 10	10	10	10 1	0 10	10	10	10	10 1	0 1	.0 1	0 1	0 1	0 1	0 1	0	T			T		T
~	Santa Anita SB	31.73		+	T	T		T			-											10 1																				T	T		T	T	T
₿8	Santa Anita NB	31.91											10	10	10	10	10	10	10	10 :	10 :	10 1	0 1	0 1	0 10	0 10	10	10	10 1	0 10	10	10	10	10 1	0 1	.0 1	0 1	0 1	0 1	0 1	0				T		T
-210	Huntington	32.76		+	t	T	-	t	+		-		10	10	10	10	10	10	10	10 :	10 :	10 1	0 1	0 1	0 10	0 10	10	10	10 1	0 10	10	10	10	10 1	.0 1	.0 1	0 1	0 1	0 1	0 1	0	T	T		T	T	T
7	Myrtle	32.76		+	t	t		t			-											10 1																				T	T		T	T	t
	Mountain	34.61		-	+	+	-	+	-	-	-											10 1																									T
	Buena Vista	35.12			t	T		T														10 1																				T	+	T	T	T	t
	I-605 Connector	36.00			T	T	T	T													-	32 3													T		T	T	Т	T		+	T	-	T	1	T
	Mount Olive	36.30			t	t		T														10 1													1		t	1	+	t	T	T	+	T	T		t
	Irwindale SB	37.78			t	T		t			1											10 1								1							T		+	+	+	+	+	-	1	1	t
	Irwindale NB	37.92		t	t	t		t														10 1															t		1	+		1	+		1	1	t
	Vernon	38.87					-		-	+	-	-	-	-	_	_	_	_	_		_	10 1		_	-	-		+	+	-	-		-	-	+	-		+	-	+	-	-	+	-	-	-	t
	Azusa SB	39.52		t	t	t		t														5													+		t	+	+	+	+	+	+	+	-	+	t
	Azusa NB	39.62			t	+	+	+	-	-												5 10 1			-				+						+		+	+	+	+	+	+	+	+	+	-	t
	Citrus NB	40.70	-	-	+	t	-	+	+	+	-	-						_				10 1		_	+	-		-	+	+	-		-	+	+		+	+	+	+	+	+	+	+	+	+	+
	CICICID IND	40.70											10	10	10	10	10	10	10	10 .	10 .	10 1	0 1																								110













## Ramp Metering – Modeling Considerations

- Custom APIs required for modeling non-basic ramp metering operations
  - Queue overrides
  - Mainline congestion override
  - Dynamic metering algorithms
    - Linear Mainline Responsive (LMR)
    - SWARM
    - Fuzzy Logic
    - HERO
    - Etc.

















## **Transit Modeling Needs**

### Simulate impacts on vehicular traffic

- Bus stopping on road
- Signal priority/preemption
- Simulate impacts of incidents on bus operation
  - Delays due to congestion
- Simulate responses to incidents with transit component
  - Changes in transit service

















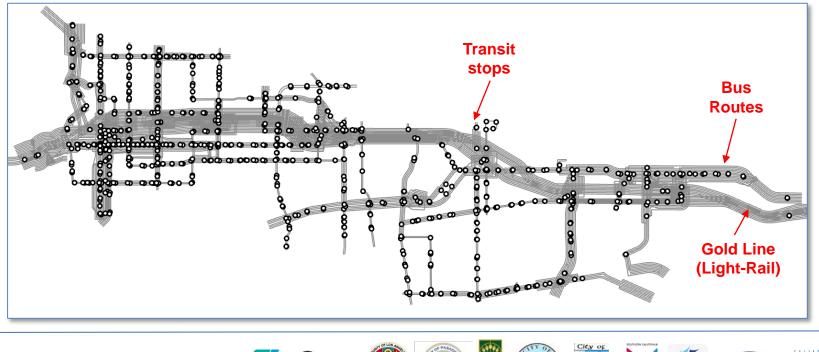


### **Transit Services Modeled**

#### Metro Gold Line

#### Express and local bus routes

 Metro Bus (18), Foothill Transit (5), LA DOT (1), Pasadena Transit (9) Arcadia Transit (3) and Duarte Transit (2)



Metro





## **Transit Modeling**

#### Location of bus stops

#### □ Types of stop

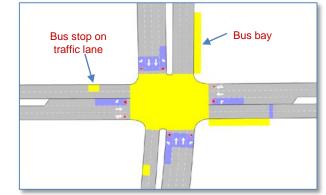
- Bus stopping in the roadway
- Bus bays

#### Service timetables

- Weekday / Saturday / Sunday departure tables
- Vehicle type used on each run

#### Dwell times

- Average time stopped at each service point
  - 20 s average duration
  - 10 s variance
- Can be adjusted where field observations are available



etable: Weekday									ment procession
Contraction in the second s							<ul> <li>✓ New</li> </ul>	Delete	Duplicat
chedules				Departure					
Initial Time	Duratie		arture Times	1000000	leparture Tim	Deviation	1.40 a Posto	ink Delay Tim	a ranati
08:44:00	00:37:00	Interval (P	unctuall		20	00:00:00	None	00:00:00	INK PHISE VEHIC
09:30:00	01:00:00	Fixed		Light Rail T					
18:45:00	01:00:00	Fixed		Light Rail T	09:57:00	00:00:00	None	00:00:00	
23-27-00	01:41:00	Interval (P	unctual)	Light Rail T	10:15:00	00:00:00	None	00:00:00	
				Light Rail T	10:27:00	00:00:00	None	00:00:00	
New	Delete								mark
Show Pedestrian Info								New	Delete
Dwell Times									
Divide Times							10	Offset (s)	^
Sto	p	Mean (	)		Dev.		-	UTISEL 15/	
-		Mean (	a	10.0	Dev.	0.		011561 (5/	
Sto	in NB 3		a	10.0	Dev.	ō. 0.	٥	011361157	-
Stor 7633903: Fillmore Static	n NB 3	0.0	0		Dev.		0	Unserts/	
Sto 7633903: Fillmore Static 7633884: Del Mar Static	n NB 3 n NB 3 c Station NB 3	0.0	0	10.0	Dev.	0.	0 0 0	01136132	













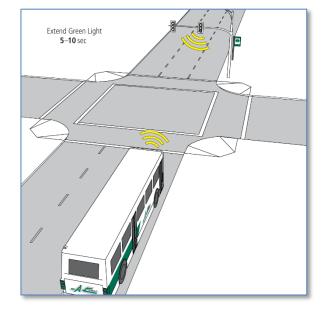
## Transit Signal Priority/Preemption

### Definitions

- Preemption: Force change in signal operations
- Priority: Change in signal operations if signal operational constraints allow it

### Modeling needs

- Simulation of signal preemption required at light-rail crossings to adequately capture corridor operations
  - 11 at-grade crossings
  - Interruptions every 3 minutes during peak hours
- Corridor stakeholders interested in testing bus priority at several intersections















## Transit Signal Priority/Preemption

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### Existing/proposed intersections with priority/preemption control





## **Transit Signal Priority/Preemption**

#### **Modeling tasks**

- Basic priority logic available in Aimsun
- Light-rail preemption not available
  - Can use existing logic as approximation
  - Cannot force signal to terminate before minimum green ends
  - Need to develop an API

	Sana Cha					Santa Clarp Santa Clarp
ype: Actuated tings: 1 + Timing Pre-en	nption		ec 🗘 Cyde: 75 s			
	Public Transit Pre-emption 1		•	Add	Delete	
Public Transit Lin ID 7633945 7638098 7633913	Add Gold Line NB (Azusa) Gold Line NB (Mon Gold Line NB (Sierr	Phases	Name		Add Delete	
Priority Request	Detectors	Priority End De	etectors			
ID 7638038	Name Gold Line - Santa C Delete	ID 7638042	Name Gold Line - S	Santa C	Add Delete	
Parameters						
Delay:	0	Inhibit:	0			
Minimum Dwell:	6	Maximum Dwell:	29			
Reserve:	0	Type:	Alternative		•	
		_	_	_		

















## **Calibration Objective**

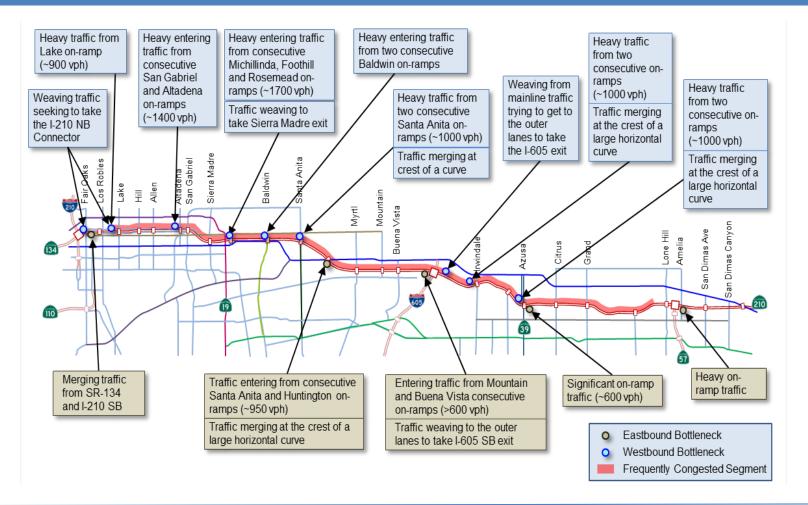
- Simulation of representative traffic volumes on freeway and arterials for time period considered
- Replication of bottlenecks on freeway
  - Location
  - Speed
  - Extent

### Replication of observed queuing at intersections

- Observed approach speeds
- Location of queues
- Queue extent



### **Operational Assessment – AM Peak**





Metro



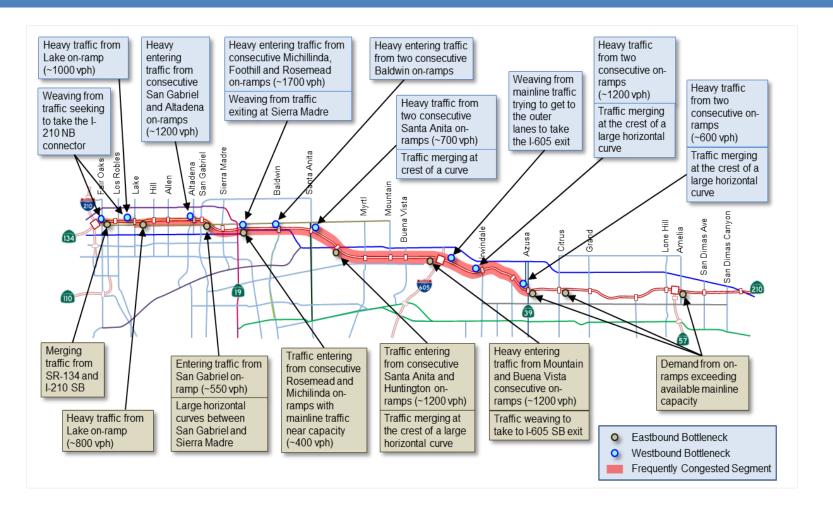








### **Operational Assessment – PM Peak**





## **Aimsun Calibration Elements**

### Vehicle characteristics

- Average vehicle length
- Driver behavior
  - Reaction time
  - Speed acceptance
  - Desired gap between vehicles
  - Aggressiveness in accepting short gaps
  - Lane change cooperation
  - Braking intensity
  - etc.

1ain	Slope	Lanes	Dynamic Models	Static M	odel	Usage	Attributes		
Attrac	tiveness:			•	5500	.00			-
Micro									
Lan	ie Changin	ng				Side La	ine		
Coo	operation:		80.00 %		•	Coope	ration Distance:	Whole Lane	*
Agg	gressivene	ess:	0.00 %		•	Mergin	g Distance:	Default	* *
Bra	king Inter	sity:	Normal		•	🗹 Me	rge: First Vehide	On is First Vehicle O	ff
	Impruden	nt Lane Cha	nging						
Que	eue Discha	arge				Two-W	ay Two-Lane Ov	ertaking Model	
Acc	eleration	Factor:	No	Change	•	Mirror	Section:	None	$\sim$
Add	ditional Re	action Time	at Stop: 0.	00 sec	-	Visibilit	y Distance:	984.25 ft	*
Add	ditional Re	action Time	at Traffic Light: 0.	00 sec	•	Visibilit	y Factor:	1.50	÷
нст	M Settings	(Category	: None)			Two-La	ne Car-Following	Model	
	Weaving		Merge / Diverg	e Starting P	Point	<b>⊘</b> Co	nsider Two-Lane	Car-Following Model	
Meso									
Jam [	Density (p	er lane):	190.00 veh/mi		≑ Re	eaction Tim	e Factor:	1.00	*
Lan	e Selectio	n Model							
	Penalice 9	Shared Lan						Penalise Slo	wlanes













## **Calibration Data Sources**

#### Data from traffic sensors

- Traffic volumes
- Turning counts
- Observed speeds
- Detector occupancy

#### Data from traffic studies

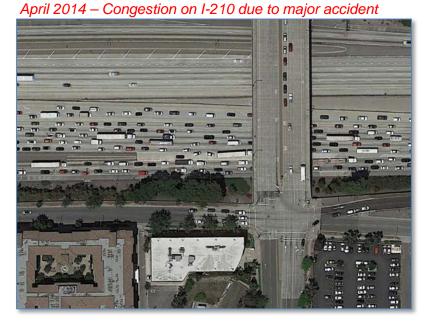
- Observed volumes
- Turning counts

#### Aerial imagery

- Vehicle length
- Spacing between vehicles

#### Probe vehicle data

- Travel time data
- Observed paths















## **Existing Calibration Guidelines**

- Existing guidelines for calibrating microscopic models
  - Cover relatively well calibration of freeways
  - Easily applied to small networks

#### FHWA Calibration Guidelines

Measure	Calibration Criteria	Acceptance Target
Modeled	Individual link flows:	
link flows	<ul> <li>Flow within 100 vph for links with &lt; 700 vph</li> </ul>	> 85% of cases
	• Flow within 15% for links with 700 to 2700 vph	> 85% of cases
	• Flow within 400 vph for links with $> 2700$ vph	> 85% of cases
	<ul> <li>GEH statistic &lt; 5</li> </ul>	> 85% of cases
	Sum of all link flows:	
	• Flow within 5%	For all link counts
	• GEH < 4	For all link flows
Modeled	Journey times within network:	
travel	• Within 15% or 1 minute, whichever criterion is	> 85% of cases
Times	higher	
Visual	Individual link speeds:	
Audits	<ul> <li>Visually acceptable speed-flow relationships</li> </ul>	To analyst's
	Bottlenecks:	satisfaction
	<ul> <li>Visually acceptable queuing</li> </ul>	To analyst's
	, , ,	satisfaction

$$GEH = \sqrt{\frac{(E-V)^2}{0.5 (E+V)}}$$

E = Model estimated volumes V = Field counts



## **Existing Calibration Guidelines**

### Items to consider

- Vagueness of "to analyst's satisfaction" criterion
- Availability of reliable data to support the calibration
  - Problem particularly acute for arterials
- Inherent variability of arterials traffic
  - Traffic entering/leaving arterials between intersections
  - Natural variability of arterial traffic flows
  - Ability for motorists to easily change route



## **Emerging Guidelines: Cluster-based Analysis**

### Control for time-variant outliers

Simulated day stays within a confidence interval defined by the cluster

### Control for time-variant "inliers"

- Match spatio-temporal critical points such as
  - Iowest observed speeds
  - outflow at active bottlenecks

### Bounded dynamic absolute error

Average error between simulated and representative days should be less than error between the representative day and all days in cluster

### Bounded systematic error

Simulated day does not provide systemically biased results



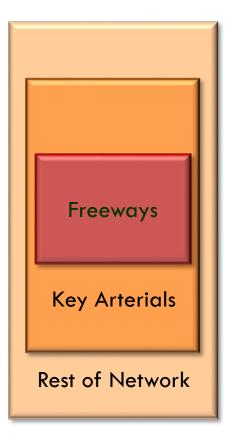
## **Calibration Tiers**

### Detailed calibration of freeways

- Flows and speeds on mainline/HOV sections
- Major bottlenecks
- Ramp queues

### Reasonable calibration of key network arterials

- Flows and speeds on arterial segments
- Turning proportions at key intersections
- Queues at key intersections
- Rough calibration of arterials at edge of network
  - Observed flows
  - No unusual congestion at main intersections



### Freeway elements

Measure	Calibration Criteria	Acceptance Target
Link flows	Individual link flows: • Links with < 700 vph → Within 100 vph • Links with 700 to 2700 vph → Within 15% • Link with > 2700 vph → Within 400 vph • GEH statistic < 5 Sum of all link flows: • Total flow within 5% • GEH < 4	> 85% of links > 85% of links > 85% of links > 85% of links Over all links Over all links
Travel Times	<ul> <li>Travel times along key freeway segments:</li> <li>Within 15% or 1 minute, whichever is higher</li> </ul>	> 85% of cases
Recurrent Bottlenecks	Location: • Front within 0.50 mile • Extent within 0.50 mile Time of occurrence: • Start time within 30 min of observed start • End time within 30 min of observed end	> 85% of cases* > 85% of cases* > 85% of cases* > 85% of cases*

$$GEH = \sqrt{\frac{(E-V)^2}{0.5 (E+V)}}$$

E = Model estimated volumes V = Field counts

QUESTION: most freeway segments in corridor carry 6000-7000 veh/hr  $\rightarrow$  400 veh/hr criterial would impose a 5-6% max error

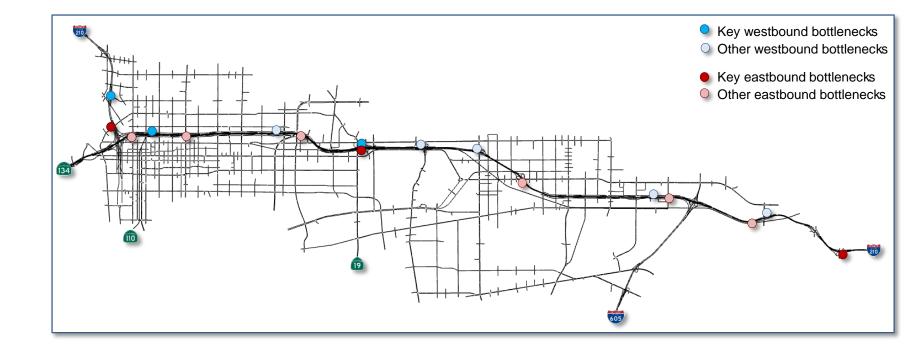
→ Acceptable?

\* All key major bottlenecks must fall within the 85% accepted cases



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### Main corridor freeway bottlenecks





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#### **Arterials**

Measure	Calibration Criteria	Acceptance Target
Link flows	Individual link flows: • Links with < 700 vph → Within 100 vph • Links with 700 to 2700 vph → Within 15% • Link with > 2700 vph → Within 400 vph • GEH statistic < 5 Sum of all link flows: • Total flow within 5% • GEH < 4	> 85% of links > 85% of links > 85% of links > 85% of links Over all links Over all links
Turning Proportions	<ul><li>Turning percentages on intersection approaches</li><li>Within 25% of observed percentages</li></ul>	> 85% of cases*
Travel Times	Travel times between key intersections <ul> <li>Within 15% or 1 minute, whichever is higher</li> </ul>	> 85% of cases*
Congested Intersections	<ul> <li>Location:</li> <li>Queuing at known congested intersections</li> <li>Extent of queue between typical intersections</li> <li>Time of occurrence:</li> <li>Start time within 30 min of observed start</li> <li>End time within 30 min of observed end</li> </ul>	> 85% of cases* > 85% of cases* > 85% of cases* > 85% of cases*

 $GEH = \sqrt{\frac{(E-V)^2}{0.5 (E+V)}}$ 

E = Model estimated volumesV = Field counts

\* Over key major intersections







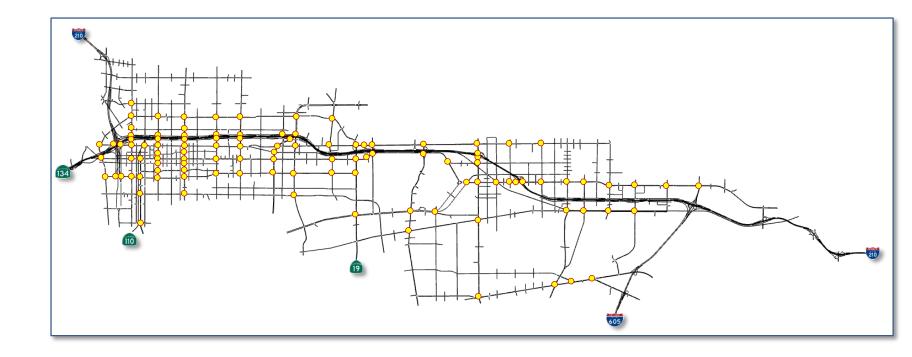






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### Key arterial intersections (preliminary list)





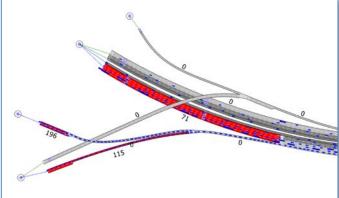
## Calibration Steps

#### **Step 0** – Error and Validity Testing

- Single run tests
  - Check for ODs with non-existent paths
  - Reasonable network loading and emptying
- Stress tests
  - Unreasonable congestion
  - Virtual queues
  - Missed turns
  - Reasonable route choices

Missed Turns Analysis

#### Virtual Queue Analysis























## **Calibration Steps**

### Step 1 - Recreate congestion using constant average demand for simulation period

- Appropriate vehicle routing decisions
- Bottlenecks occurring at right location
- Separate analyses for AM and PM peaks

### Step 2 - OD matrix adjustment based on observed data

- Adjust Car OD matrix using count and turn data
- Adjust HOV OD matrix using HOV specific data only

# Step 3 – Recreate congestion using demand profile(s) for each simulation period

Focus on congestion onset, extent, and dissipation



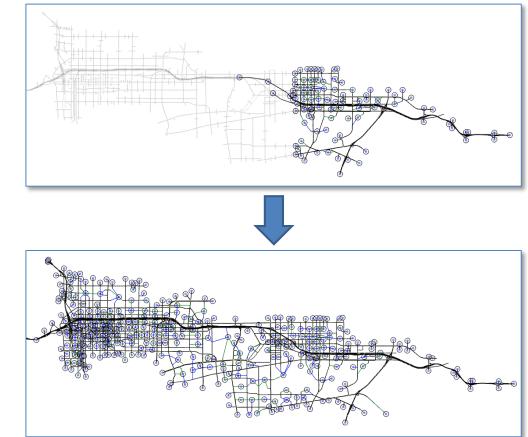
## **Calibration Steps**

### Preliminary calibration using small area

- Saves time by allowing quicker simulation than full network
- Global parameters
- Template for freeway merge/diverge areas
- Template for congested intersections

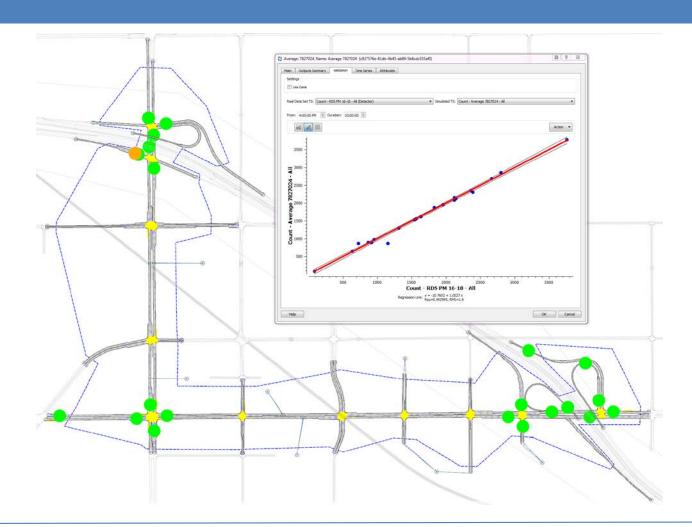
#### Full network calibration based on results of small calibration effort

- Minor adjustment of global parameters / templates
- Calibration of local congestion hotspot





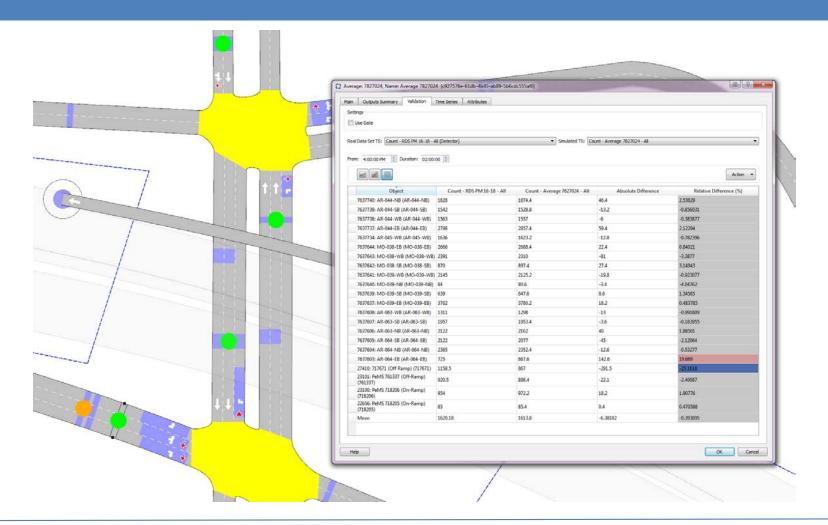
### **Example Calibration: Santa Anita Reroute**





## **Example Calibration: Santa Anita Reroute**

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# **Initial State Estimation**

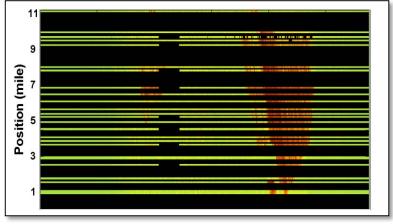
#### Information available from input data streams

- Vehicle flows on instrumented segments/approaches
- Vehicle speeds at various locations
- Travel times between specific locations
- Detector occupancies
- Need: Representation of current traffic conditions within the corridor that can be used as a starting point for a simulation

#### Modeling considerations

- How to fill in information gaps?
- Using a continuous simulation run to produce initial states does not prevent divergences between reality and simulated conditions







Method needed to develop estimates of current traffic states and input these states into Aimsun













# Initial State for Aimsun Traffic Prediction

#### Inputs

- From off-line simulation runs of calibrated model:
  - Library of initial states
  - Distribution of destinations from each link, weighted by flow, obtained via traffic assignment
- From on-line streaming data:
  - Estimated traffic state on freeway segments
  - Estimated traffic state on arterial routes

#### Adjustment procedure

- Modify candidate initial state produced by Aimsun by adding/removing vehicles from each link to match estimated number of vehicles
- Adjust placement to represent vehicles in queue or approaching a queue

UC Berkeley and TSS are currently working together to implement this functionality



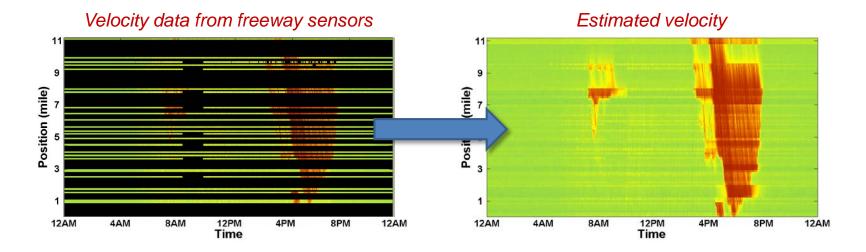




### **Result of Freeway Estimation**

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- Real time data exist at specific points along the road
- Estimation fills in the blanks to provide a complete picture of traffic state





### **Freeway Traffic Estimation**

#### Goal

Provide a complete picture of traffic conditions along a freeway based on observed data

#### Input

- Network of roads represented as links and nodes
- Fundamental diagrams for each link
- Boundary flows at edges of network
- Turning movements (split ratios) at each node 🚔
- Real-time flows and occupancies from detectors

#### Output

Velocities and densities on each link

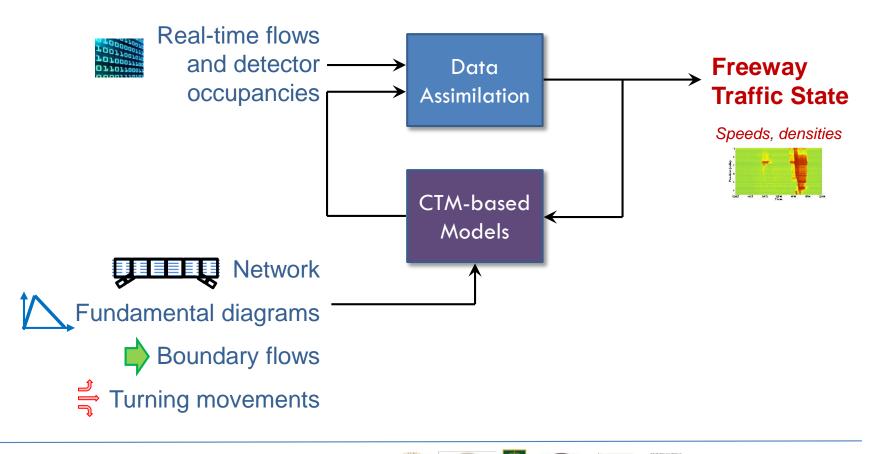






### **Freeway Traffic Estimation**

#### Estimation process



Foothill Transit

Metro

# Calibration of Freeway Estimator

- Calibration for estimation is much easier than calibration for prediction
- Two key parameters
  - Data noise variance
  - Assimilation "process noise" variance

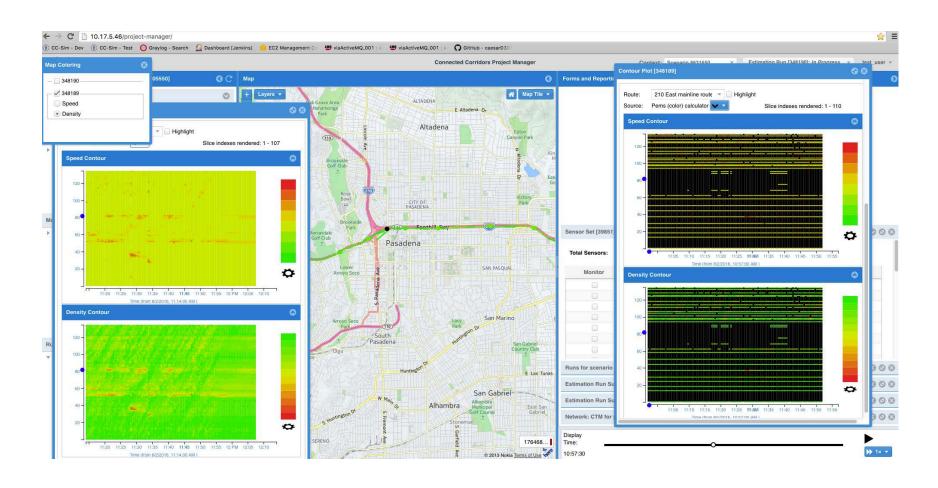
#### Leverage data quality efforts

- Fundamental diagrams, turning movements, and boundary flows measured directly from data
- Good data  $\rightarrow$  good estimation results
- Estimation fills in the blanks, so filter aggressively to remove suspect data





### Estimation Running in the Cloud





CALIFORNIA

Foothill Transit



### Arterial Traffic Estimation

#### 🗆 Goal

 Estimation of traffic conditions on arterial segments at a given time based on observed data

#### Input

- Intersection geometry
- Signal timing plans
- Historical approach flows and turning counts
- Real-time sensor counts and occupancies from advance and stop line detectors

#### Output of current process

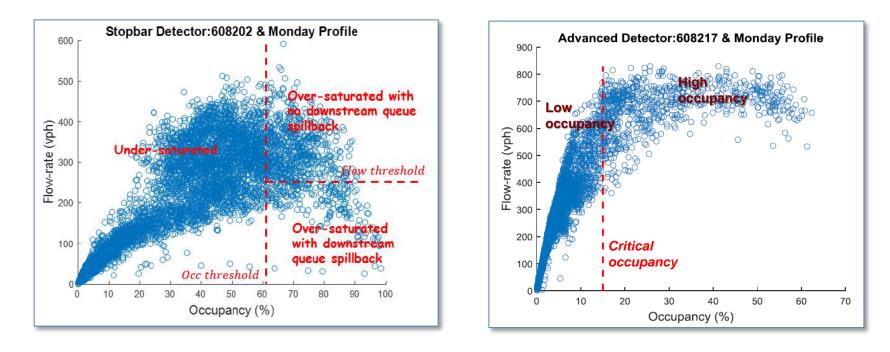
 Average queue lengths for each turning movement at individual intersections



### **Classification of Detectors**

#### Different thresholds to classify traffic conditions for different types of detectors

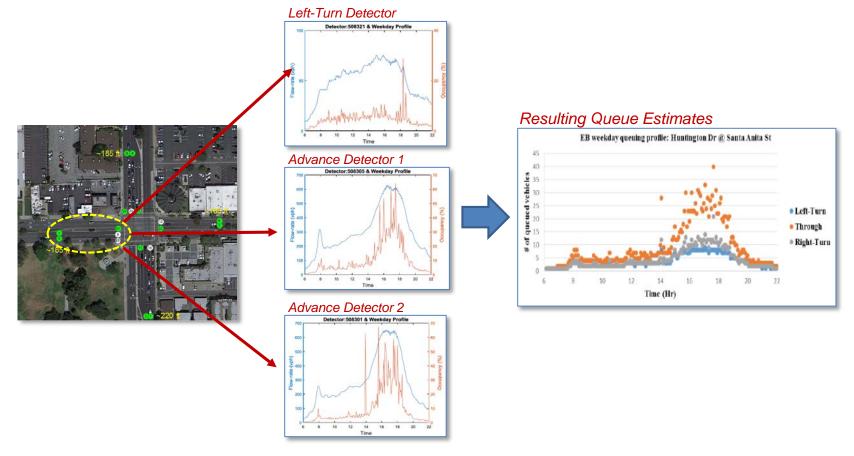
- Two thresholds for stop line detectors: detector occupancy and flow
- One threshold for advanced detectors: critical detector occupancy





### **Queue Estimation**

Example: estimated queues for left-turn, through, and right-turn movements 













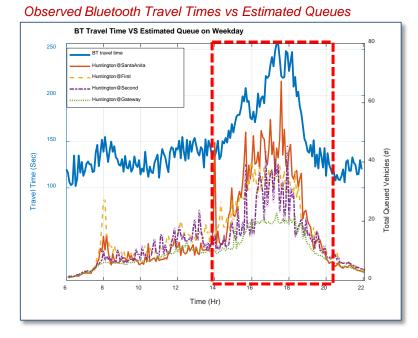


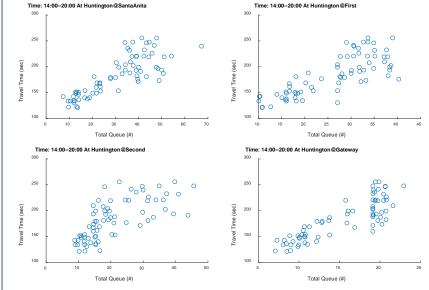




### **Arterial Traffic Estimation**

#### Estimation results consistent with Bluetooth travel times





#### Relationship between Observed Travel Times and Estimated Queues



### **Calibration of Arterial Estimator**

Simple arterial model with nothing to tune

#### Thresholds are calculated directly from data

- Two thresholds for stop line detectors: detector occupancy and flow
- One threshold for advanced detectors: critical detector occupancy

#### Leverage data quality efforts

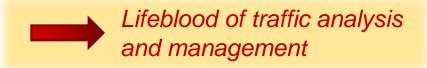
- Good data  $\rightarrow$  good estimation results
- Filter aggressively to remove suspect data





# Why is data quality so important?

Importance of high-quality data—including its timeliness, accuracy, and coverage—cannot be overstated



- Quality of work depends directly on quality of data
  - Missing data → reduced situational awareness
    - Unable to locate routes with available capacity
  - Bad data → bad decisions
    - → Bad management and worse traffic
    - $\rightarrow$  Increased risk to pilot deployment



### Data Quality Considerations

#### Basic detector health

- Do we know and agree where the sensors are?
- Are the sensors labeled and configured correctly?
  - No sensors on the wrong side of the freeway
  - No HOV sensors mistaken for ML sensors, etc.
- Do the sensors capture a full cross section of traffic flow?
- Are the sensors turned on, and communicating data regularly?

#### Data Accuracy

- Is the data provided by a detector trustworthy?
- When we compare traffic flowing into and out of each section of freeway, do the numbers make sense (flow balance)?
- Are data consistent with traffic engineering expectations?











### Assessing Freeway Detector Health

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#### Weekly freeway detector health status report based on PeMS data

Weekly Average Data		Eastbound I-210 PM 25 to PM 43.25									
Quality	CD	СН	Fwy-Fwy	HOV	Mainline	Off Ramp	On Ramp	Total			
May 1-7	n.a.	n.a.	100.0%	69.4%	78.4%	73.3%	86.3%	77.8%			
May 8-14	n.a.	n.a.	100.0%	81.6%	87.9%	73.3%	82.1%	85.0%			
May 15-21	n.a.	n.a.	100.0%	83.3%	87.4%	72.4%	83.9%	84.9%			
May 22-28	n.a.	n.a.	100.0%	74.7%	80.6%	73.3%	89.9%	80.2%			
May 29 - Jun 04	n.a.	n.a.	100.0%	66.1%	70.7%	68.6%	81.0%	71.5%			
Jun 05-11	n.a.	n.a.	100.0%	78.0%	82.4%	70.5%	82.1%	80.7%			
Jun 12-18	n.a.	n.a.	100.0%	85.3%	90.2%	73.3%	89.3%	87.5%			
Jun 19-25	n.a.	n.a.	90.5%	80.0%	87.2%	72.9%	88.1%	84.5%			
Jun 26-Jul2	n.a.	n.a.	66.7%	72.2%	81.5%	73.3%	91.7%	79.8%			
Jul3-9	n.a.	n.a.	66.7%	70.2%	79.2%	70.0%	89.3%	77.4%			
Jul10-16	n.a.	n.a.	64.3%	78.0%	87.1%	68.6%	92.3%	83.4%			
Jul17-23	n.a.	n.a.	47.6%	81.2%	87.7%	71.4%	93.5%	84.4%			
Jul24-30	n.a.	n.a.	61.9%	75.1%	80.2%	60.0%	74.4%	76.0%			
Jul31-Aug06	n.a.	n.a.	33.3%	77.6%	82.2%	64.3%	82.7%	78.2%			
Aug07-Aug13	n.a.	n.a.	33.3%	82.9%	87.7%	70.0%	92.3%	84.0%			
Aug14-Aug20	n.a.	n.a.	33.3%	78.4%	85.8%	71.9%	87.5%	81.9%			
Aug21-Aug27	n.a.	n.a.	33.3%	86.5%	90.5%	78.6%	92.9%	87.3%			
Aug28-Sept3	n.a.	n.a.	33.3%	86.5%	91.1%	78.1%	92.3%	87.5%			
Sept4-Sept10	n.a.	n.a.	33.3%	84.5%	90.5%	73.8%	91.1%	86.2%			
Sept11-Sept18	n.a.	n.a.	33.3%	86.5%	91.4%	78.1%	89.9%	87.5%			
Sept18-Sept25	n.a.	n.a.	33.3%	87.8%	92.7%	81.0%	91.7%	88.9%			
Sept25-Oct2	n.a.	n.a.	33.3%	84.9%	90.3%	74.3%	91.1%	86.2%			
Loops in Category	0	0	6	35	148	30	24	243			















### Assessing Freeway Detector Health

- Daily check of detector flow data for consistency
- Diagnostics to assist with identification of
  - Missing data
  - Problematic sensors,

#### Follow up fix requests

ID	Fwy	Name	Туре	1-Jun	2-Jun	3-Jun	4-Jun	5-Jun	6-Jun	7-Jun	8-Jun	9-Jun	10-Jun	11-Jun	12-Jun	13-Jun
716578	I210-W	LINCOLN 1	On Ramp	No data												
763911	I210-W	LINCOLN 2	Off Ramp	Ok												
770568	I210-W	HAMMOND	Mainline	Others	Dir	Others										
717624	I210-W	MOUNTAIN	Mainline	No data												
717623	I210-W	MOUNTAIN	Off Ramp	Ok												
716579	I210-W	MOUNTAIN	On Ramp	Ok												
770579	I210-W	WINONA WA	Mainline	Dir												
770165	I210-W	EB 134 TO W	On Ramp	Others												
770157	I210-W	EB 134 TO W	Mainline	No data												
716582	I210-W	WALNUT	On Ramp	No data												
		WALNUT	Mainline													
769300	1210-W	WB 210 TO O	Off Ramp	Others	Others	Others	Others		Others							
717630	I210-W	FAIR OAKS 1	Mainline	Both	HOV	HOV	Dir	HOV	HOV	HOV	HOV	Both	Ok	Ok	Dir	HOV
716583	I210-W	FAIR OAKS 1	On Ramp	Ok												
717632	I210-W	FAIR OAKS 1	HOV	Both	Both	HOV	Ok	HOV	HOV	HOV	HOV	HOV	Ok	Ok	Ok	HOV
773132	I210-W	FAIR OAKS O	Off Ramp	Ok												
764137	1210-W	MARENGO	Mainline	Ok												
764135	I210-W	MARENGO	HOV		Ok											
764349	I210-W	MARENGO	Off Ramp	Ok												
	1210-W		On Ramp		Ok											
717634	I210-W	LAKE 1	Mainline	Ok												
761318	I210-W	LAKE 1	HOV	Ok												













### Fixing Freeway Detectors

# Example: Fix requests for detectors assigned to wrong side of freeway or wrong lane

Fwy City		Abs PM ID		Name	Туре	Issue to be addressed
		40,189				
I210-E	0-E Azusa		772905	PASADENA AVE	HOV	Wrong side of fwy
I210-W	Azusa	40.189	772904	PASADENA AVE	HOV	
I210-E	Azusa	40.189	772903	PASADENA AVE	Mainline	Wrong side of fwy
I210-W	Azusa	40.189	772902	PASADENA AVE	Mainline	wrong side of twy
1605-N	Irwindale	26.552	773795	ARROW HIGHWAY	Mainline	Wrong side of fwy
1605-S	Irwindale	26.552	773796	ARROW HIGHWAY	Mainline	
SR134-W	Los Angeles	Los Angeles 11.623 774032 COLORADO Mainlir		Mainline	GP/HOV swap	
SR134-W	Los Angeles	11.623	774034	COLORADO	HOV	GF/HOV Swap
I210-E	Azusa	39.929	765477	AZUSA 1	Mainline	
I210-E	Azusa	39.929	770407	AZUSA 1	HOV	GP/HOV swap
I210-W	Duarte	35.409	761371	BUENA VISTA	HOV	
I210-W	Duarte	35.409	761374	BUENA VISTA	Mainline	GP/HOV swap
I210-W	Arcadia	30.999	717665	BALDWIN 2	HOV	
I210-W	Arcadia	30.999	717664	BALDWIN 2	Mainline	GP/HOV swap
I210-W	Arcadia	30.139	717661	MICHILLINDA	Mainline	
I210-W	Arcadia	30.139	761327	MICHILLINDA	HOV	GP/HOV swap
I210-W	Pasadena	28.27	717645	SAN GABRIEL	HOV	Ramp configured as HOV



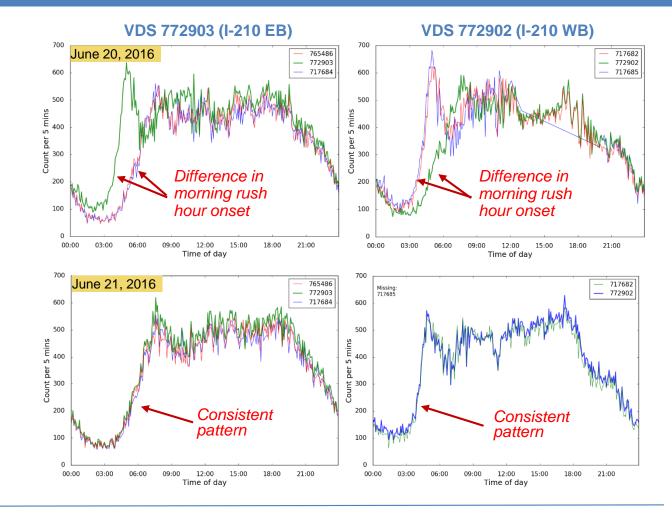








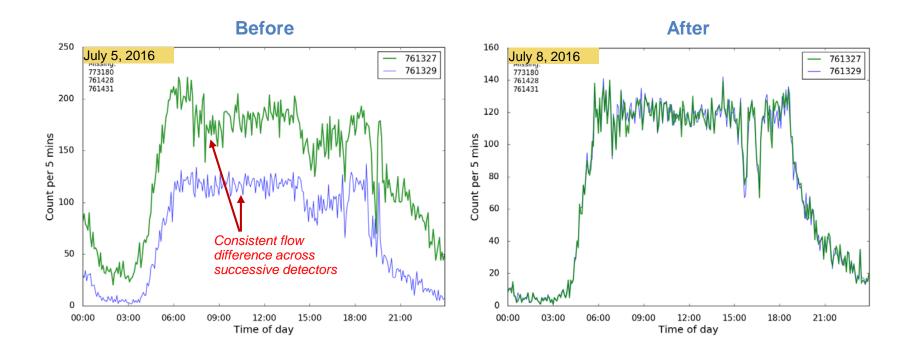
 Detectors assigned to wrong side of freeway





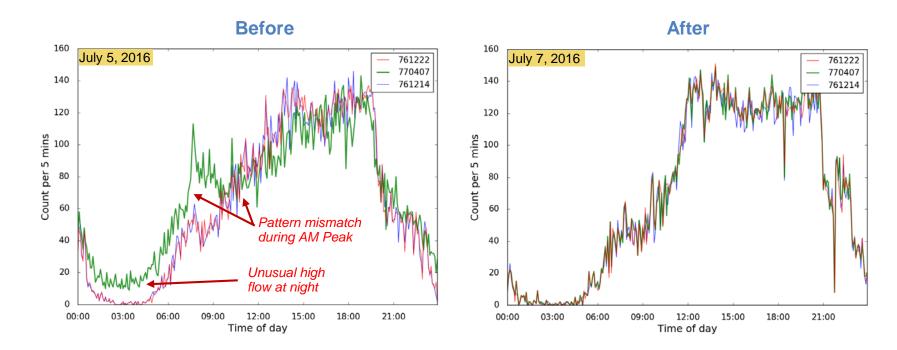


I-210 WB HOV lane detector at Michillinda: Incorrect HOV/general purpose lane assignments





I-210 EB HOV lane detector at Azusa 1: Incorrect HOV/general purpose lane assignments



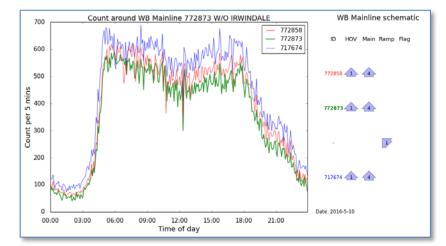


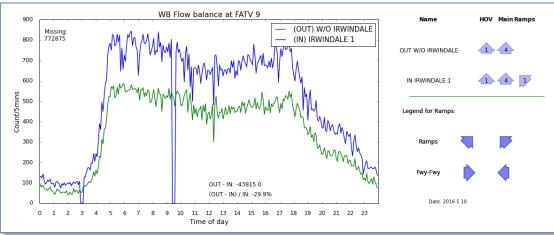
# I-210 WB west of Irwindale: Flow undercounting

30% observed balance flow different between successive stations

balance error is not possible with just a missing HOV lane alone (772875).

Likely due to missing lane data

















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#### VDS 772873 on I-210 WB west of Irwindale

A lactions	<b>C</b> PeMS	15.1							
	Mainline VDS 77287	3 - W/O IRWI	NDALE						
and the second	Current Location		Change Log	Performance 🔻	Data Quality 🔻	Events 🔻			
40	+		Change Lo	og					
	0		Roadway In Road Width Lane Width	formation (fro	om TSN) 72 ft 12.0 ft				
	210 F	oothill Fwy	Inner Shoulde	r Width r Treated Width	2 ft 2 ft				
Foothill Fwy			Outer Shoulde Outer Shoulde Design Speed I	r Treated Width	10 ft 10 ft 70 mph				
		Leaflet   © HERE	Functional Clas	55		rial W/ C/L Prin Arterial badway Use			
	Maps <u>Real-Time</u> Performance In	iventory	Inner Median V Terrain Population	Width	30 ft Flat Urbanized	Flat			
O E	I210-W @ CA PM R37.5 (Al District 7, Los Angeles Cou Irwindale	Barrier Surface		Concrete Barrier Bridge Deck					
	Station Details		Roadway Use		Median Lane	is HOV Lane			
5 lanes in reality	Aliases LDS	MS ID 2120 772861	Change Log Date		Status	Name	Lan	05	
	Owner	Caltrans	02/24/2010		Active	W/O IRWINDALE	1 2		
4 lanes in PeMS	Assoc. Traffic Census Station	None	09/01/2011 02/23/2012		Active Active	W/O IRWINDALE W/O IRWINDALE	1 2 1 2	3 4	
	Comm Type (LDS) Speeds	Estimated	05/02/2012 09/06/2012		Active Active	W/O IRWINDALE W/O IRWINDALE	12		
	Max Cap.	138.8 Veh/Min (04/01/201	10/13/2016		Active Active Active	W/O IRWINDALE W/O IRWINDALE W/O IRWINDALE	12	3 4	
	Vehicle Classification	N/A							



Metro











 Mismatch in number of lanes covered

Item	Fwy	VDS ID	Name	Туре	Lanes in PeMS	Lanes in Google Streetview	Comments	Findings by Caltrans
1	I210-W	770568	HAMMOND ST.	Mainline	4	5	Lane count mismatch	Outside of Corridor
2	I210-W	717107	SANTA ANITA NB	On Ramp	1	2	Lane count mismatch	1 lane 1 loop: Non-issue (8/11/16) The 2 loops are Q loops, not On loops. Location of On loop identified in photo provided (9/6/16)
3	1210-W	773194	E OF SECOND	Mainline	4	5	Lane count mismatch	Requested ML5 to be added to ATMS (8/5/16)
4	1210-W		MYRTLE AV	Off Ramp	2	1	Lane count mismatch	2 loops to cover 1 wide lane Requested FR2 to be removed from ATMS (8/11/16)
5	1210-W	769702	HIGHLAND	Mainline	4	5	Lane count mismatch	4 lanes 4 loops: Non-issue (8/11/16) Location identified in photo provided (9/6/16)
6	I210-W	773206	SB 605 FROM WB 210	Fwy-Fwy	2	3	Lane count mismatch	2 lanes 2 loops: Non-issue (8/11/16) The 3rd lane is SB 605 from Mount Olive (8/19/16) Location identified in photo provided (9/6/16)
7	I210-W	772858	SAN GABRIEL RIVER	Mainline	4	5	Tadeo is fixing	Requested ML5 to be added to ATMS (8/5/16)
8	I210-W	772873	W/O IRWINDALE	Mainline	4	5	Lane count mismatch	Requested ML5 to be added to ATMS (8/11/16)
9	I210-W	717678	AZUSA 1	Mainline	4	5	Lane count mismatch	Requested ML5 to be added to ATMS (8/23/16)
10	I210-E		BUENA VISTA	Mainline	4	5	Lane count mismatch	Requested ML5 to be added to ATMS (8/23/16)
11	1210-E		NB 605 TO EB 210 CON	On Ramp	1	2	Mismatch in PeMS only	PeMS issue (8/10/16)
12	I210-E	772857	SAN GABRIEL RIVER	Mainline	4	5	Tadeo is fixing	Requested ML5 to be added to ATMS (8/5/16)
13	I210-E	774990	IRWINDALE 1	Off Ramp	1	2	Lane count mismatch	1 loop functioning; 1 loop missing DLC (8/11/16)
14	I210-E		CITRUS 2	Mainline	4	5	Lane count mismatch	merging with an on-ramp. Will need to replace ML4 with a wider loop (8/18/16)
15	SR134-W		EB 210 TO WB 134 #2	Fwy-Fwy	1	2	Lane count mismatch	Outside of Corridor
16	SR134-E	717605	ORANGE GROVE	Off Ramp	1	2	Lane count mismatch	Outside of Corridor











#### Sensors not returning data

Item	Fwy	VDS ID	Name	Туре	MS ID	Comments	Findings by Caltrans
17	1605-S	766926	605 SB TO ARROW WB	Off Ramp	4428	Appear permanently broken	Bad loop (8/17/16)
18	1605-S	766925	605 SB TO ARROW EB	Off Ramp	4428	Appear permanently broken	Bad loop (8/17/16)
19	1605-S	773798	ROUTE 605/ ROUTE 210	Mainline	2443	Appear permanently broken	Solar panel issue (8/17/16)
20	1605-S	774260	MT OLIVE TO SB 605	Coll/Dist	4430	Appear permanently broken	Bad loop (8/17/16)
21	1605-N	773807	ROUTE 605/ ROUTE 210	Mainline	2443	Appear permanently broken	Solar panel issue (8/17/16)
22	I210-W	761322	HILL	HOV	4308	Appear permanently broken	
23	I210-W	717645	SAN GABRIEL	HOV	4306	Appear permanently broken	Bad Loop (9/30/16)
24	I210-W	717656	ROSEMEAD 2	Off Ramp	4569	Appear permanently broken	Bad Loop (9/30/16)
25	I210-W	717662	BALDWIN SB	Off Ramp	4303	Appear permanently broken	Misconfigured (9/30/16)
26	I210-W	717665	BALDWIN 2	HOV	4302	Appear permanently broken	Controller Down (9/30/16)
27	I210-W	717668	SANTA ANITA SB	Off Ramp	4301	Appear permanently broken	Misconfigured (9/30/16)
28	I210-W	764144	SANTA ANITA 2	HOV	4300	Appear permanently broken	
29	I210-W	773196	E OF SECOND	ноу	2117	Appear permanently broken	Communications Issue (9/30/16)
30	I210-W	773194	E OF SECOND	Mainline	2117	Appear permanently broken	Communications Issue (9/30/16)
31	I210-W	761350	MYRTLE AV	Off Ramp	4298	Appear permanently broken	Good-Fixed (9/30/16)
32	I210-W	772875	W/O IRWINDALE	HOV	2120	Appear permanently broken	Good-Fixed (8/12/16)
33	I210-W	761386	AZUSA 1	ноу	4290	Appear permanently broken	Conduit Damaged (9/30/16)
34	I210-W	717678	AZUSA 1	Mainline	4290	Appear permanently broken	Conduit Damaged (9/30/16)
35	I210-W	716610	AZUSA SB	On Ramp	4290	Appear permanently broken	Conduit Damaged (9/30/16)
36	I210-E	773131	FAIR OAKS OFF	Fwy-Fwy	2547	Appear permanently broken	
37	I210-E	761098	LAKE 2	HOV	4248	Appear permanently broken	
38	I210-E	769272	HILL AVE OFF	Off Ramp	2575	Appear permanently broken	Missing Cabinet, Maintenance will fix (9/30/16)
39	I210-E	716589	HILL NB	On Ramp	4249	Appear permanently broken	Fixed - Shows all Green in PeMs (9/30/16)
40	I210-E	763908	SIERRA MADRE V1	Off Ramp	2568	Appear permanently broken	SD2 change to SD3 - change form sent (9/30/16)
41	I210-E	773193	E OF SECOND	Mainline	2117	Appear permanently broken	Communications Issue (9/30/16)
42	I210-E	773195	E OF SECOND	нол	2117	Appear permanently broken	Communications Issue (9/30/16)
43	I210-E	761128	HUNTINGTON 1	Mainline	4257	Appear permanently broken	Fixed - Shows all Green in PeMs (9/30/16)
44	I210-E	761126	HUNTINGTON 1	нол	4257	Appear permanently broken	Fixed - Shows all Green in PeMs (9/30/16)
45	I210-E	761130	HUNTINGTON WB	Off Ramp	4257	Appear permanently broken	SD4 bit needs to be turned on (9/30/16)
46	I210-E	718205	HUNTINGTON WB	On Ramp	4257	Appear permanently broken	Fixed - Shows all Green in PeMs (9/30/16)
47	I210-E	761154	MYRTLE AV	Off Ramp	4259	Appear permanently broken	
48	I210-E	761167	MOUNTAIN	Off Ramp	4260	Appear permanently broken	OFF1 bit needs to be turned on (9/30/16)















# Why is data quality so important?

#### As previously indicated:

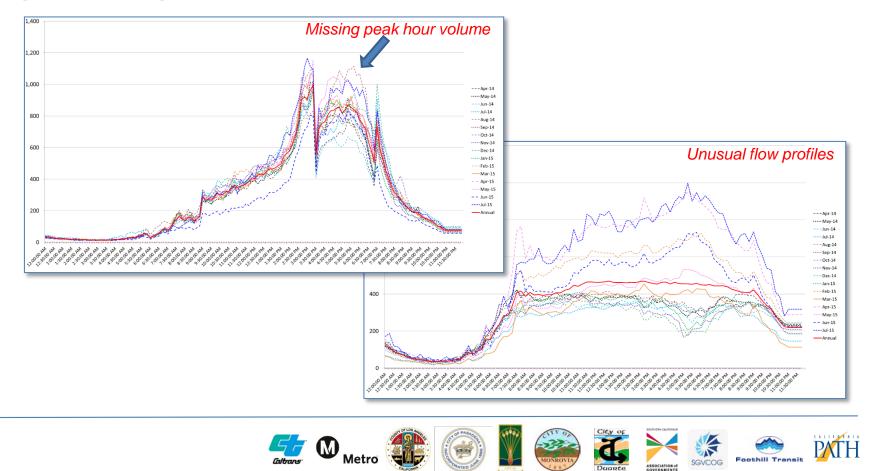
- Data is the lifeblood of traffic analysis and management
- Quality of work depends directly on quality of data

#### Key considerations

- Detector health
- Factors affecting data
- Data adjustments

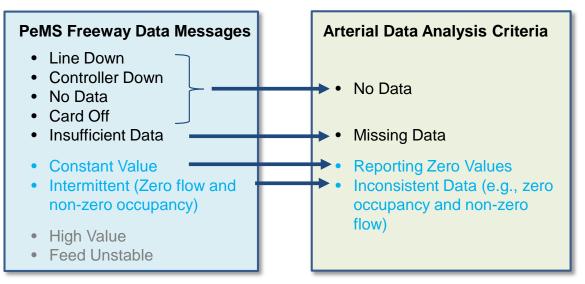


 Detector operational problems can significantly affect data produced by arterial sensors



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#### Identification of suspected errors



#### Detector categorized as "Good" if it satisfies

- Missing rate<5%</p>
- Inconsistency rate <15% (e.g., Occ =0 and flow/speed !=0)</p>
- Not reporting zero values (Major issue in Arcadia)

#### Example: System detection data from Arcadia



- ♦ System Detectors on All Approaches
- System Detectors on Some Approaches
- No System Detector Set
- Not Determined



#### **Example: Weekly detector health status report for Arcadia**

Intersections			System Dete	ctors			Arcadia								
Total	Detour	Routes	Status	T	otal	Weekly Data Quality (%)		Detour Route	es	No	t Detour Ro	utes	All Detectors		
			ON_LINE	407			Good	Bad	No Data	Good	Bađ	No Data	Good	Bad	No Data
	Yes	35	COMM ERROR	23	434	07-Feb-2016 To 13-Feb-2016	59.91	34.10	5.99	15.86	28.97	55.17	48.88	32.82	18.31
			COMM_ERROR/ON_LINE	OR/ON LINE 4		14-Feb-2016 To 20-Feb-2016	55.17	38.84	5.99	15.86	28.97	55.17	45.32	36.37	18.31
51			ON LINE	65		21-Feb-2016 To 27-Feb-2016	54.74	39.27	5.99	15.86	28.97	55.17	45.00	36.69	18.31
	No	16	COMM ERROR	76	145	28-Feb-2016 To 05-Mar-2016	50.23	43.78	5.99	15.76	29.06	55.17	41.60	40.09	18.31
			COMM ERROR/ON LINE	4		06-Mar-2016 To 12-Mar-2016	50.16	43.84	5.99	14.68	30.15	55.17	41.28	40.41	18.31
						13-Mar-2016 To 19-Mar-2016	47.43	46.58	5.99	13.79	31.03	55.17	39.01	42.68	18.31
						20-Mar-2016 To 26-Mar-2016	45.00	49.01	5.99	14.68	30.15	55.17	37.40	44.29	18.31
						27-Mar-2016 To 02-Apr-2016	46.81	47.20	5.99	14.48	30.34	55.17	38.71	42.98	18.31
riteria fo	r "Good" de	tectors				03-Apr-2016 To 09-Apr-2016	47.60	46.41	5.99	14.48	30.34	55.17	39.30	42.39	18.31
	sing rate <5%					10-Apr-2016 To 16-Apr-2016	48.52	45.49	5.99	14.48	30.34	55.17	40.00	41.70	18.31
i) Data inc	onsistency rate	e <15% (e.	g., occupancy =0, but volume !=0)			17-Apr-2016 To 23-Apr-2016	47.00	47.00	5.99	14.48	30.34	55.17	38.86	42.83	18.31
ii) Not rep	orting zero val	lues (Curre	ulty a major issue in Arcadia)			24-Apr-2016 To 30-Apr-2016	48.29	45.72	5.99	14.38	30.44	55.17	39.80	41.89	18.31
						01-May-2016 To 07-May-2016	39.10	41.47	19.42	9.16	29.26	61.58	31.61	38.42	29.98
						08-May-2016 To 14-May-2016	57.57	36.44	5.99	13.60	31.23	55.17	46.56	35.13	18.31
						15-May-2016 To 21-May-2016	60.20	33.81	5.99	14.78	30.05	55.17	48.83	32.86	18.3
						22-May-2016 To 28-May-2016	63.10	30.91	5.99	14.58	30.25	55.17	50.95	30.74	18.31
						29-May-2016 To 04-Jun-2016	61.62	32.39	5.99	13.89	30.94	55.17	49.67	32.03	18.31
	Summar	y (betweer	11-September-2016 and 24-Septer	nber-2016)		05-Jun-2016 To 11-Jun-2016	52.07	41.94	5.99	10.34	34.48	55.17	41.62	40.07	18.31
rom the w	eekly report, v	we can find	that: (i) the health rate increases signific	antly in the pa	st two	12-Jun-2016 To 18-Jun-2016	47.00	47.00	5.99	10.34	34.48	55.17	37.82	43.87	18.31
eeks, by a	about 10% at t	the network	level; (ii) the health rate in the last wee	k is currently t	he highest in	19-Jun-2016 To 25-Jun-2016	49.05	44.96	5.99	11.33	33.50	55.17	39.60	42.09	18.31
he past eigl	ht months. Suc	ch a significa	ant improvement may be related to the	maintenance se	ervice	26-Jun-2016 To 02-Jul-2016	51.38	42.63	5.99	8.97	35.86	55.17	40.76	40.93	18.31
onducted s	several weeks	ago.				03-Jul-2016 To 09-Jul-2016	51.91	42.10	5.99	8.97	35.86	55.17	41.15	40.54	18.31
						10-Jul-2016 To 16-Jul-2016	49.84	44.17	5.99	8.97	35.86	55.17	39.60	42.09	18.31
						17-Jul-2016 To 23-Jul-2016	50.53	43.48	5.99	8.97	35.86	55.17	40.12	41.57	18.31
						24-Jul-2016 To 30-Jul-2016	51.32	42.69	5.99	8.97	35.86	55.17	40.71	40.98	18.31
						31-Jul-2016 To 06-Aug-2016	50.99	43.02	5.99	8.97	35.86	55.17	40.46	41.23	18.31
						07-Aug-2016 To 13-Aug-2016	51.42	42.59	5.99	8.97	35.86	55.17	40.78	40.91	18.31
						14-Aug-2016 To 20-Aug-2016	55.92	38.08	5.99	8.97	35.86	55.17	44.16	37.53	18.31
						21-Aug-2016 To 27-Aug-2016	56.98	37.03	5.99	8.97	35.86	55.17	44.95	36.74	18.31
						28-Aug-2016 To 03-Sep-2016	53.59	40.42	5.99	11.92	32.91	55.17	43.15	38.54	18.31
						04-Sep-2016 To 10-Sep-2016	52.47	41.54	5.99	11.23	33.60	55.17	42.14	39.55	18.31
						11-Sep-2016 To 17-Sep-2016	61.95	32.06	5.99	16.06	28,77	55.17	50.46	31.24	18.31
						18-Sep-2016 To 24-Sep-2016	63.79	30.22	5.99	16.55	28.28	55.17	51.96	29.73	18.31











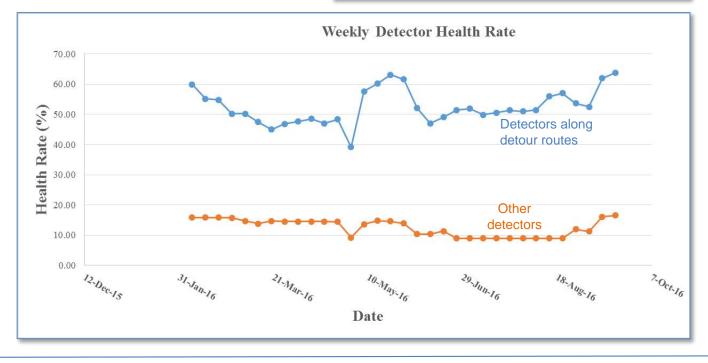




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#### Example: Weekly detection health summary for Arcadia

l.	ntersection	s	System Detectors					
Total	Detour Routes		Status	Total				
	Yes		ON_LINE	407				
		35	COMM_ERROR	23	434			
51			COMM_ERRORION_LINE	4				
51		16	ON_LINE	65				
	No		COMM_ERROR	76	145			
			COMM_ERRORION_LINE	4				

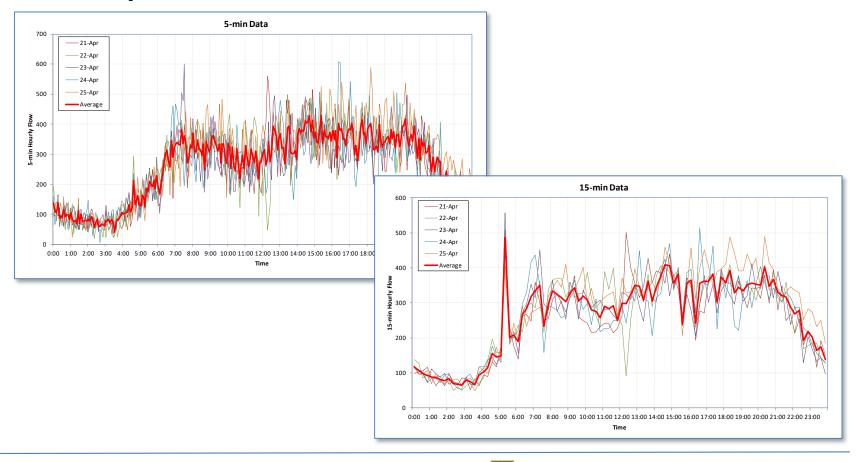




### Data Factors – Data Variability

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#### Variability of arterial traffic flows



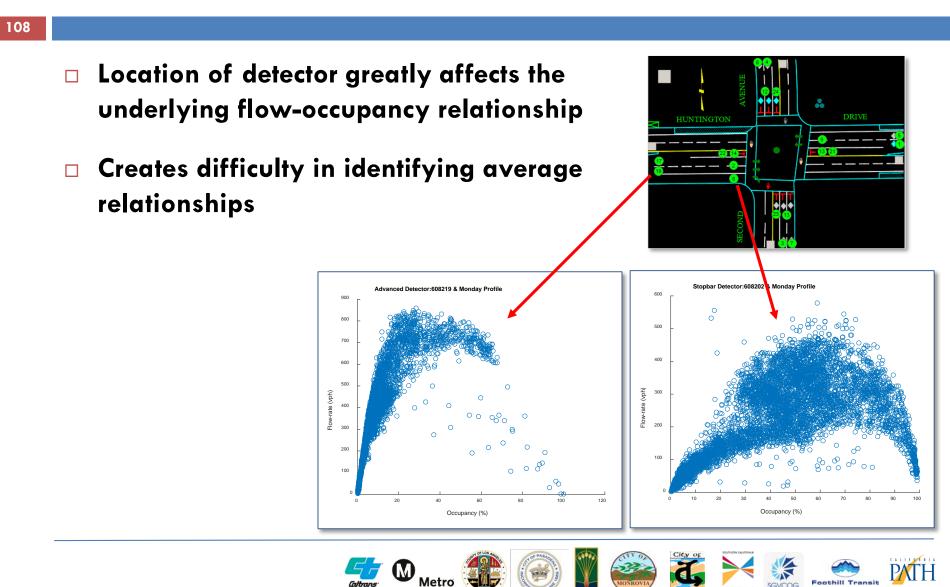
CALIFORNIA

Foothill Transit

GVCOG



### Data Factors – Flow-Occupancy Relationships



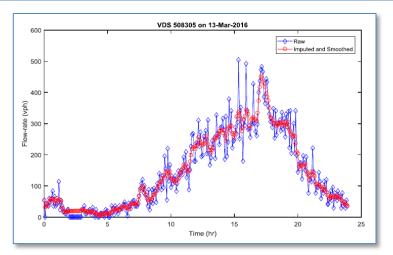
# Data Adjustments – Smoothing

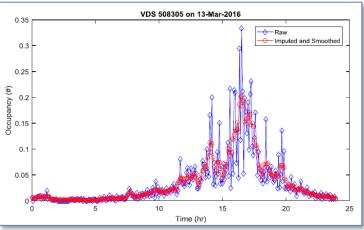
### ssue:

High degree of fluctuation, particularly when dealing with short intervals (cycle data, 5-minute data)

#### Solution:

Calculate local averages with a window span of 5 intervals































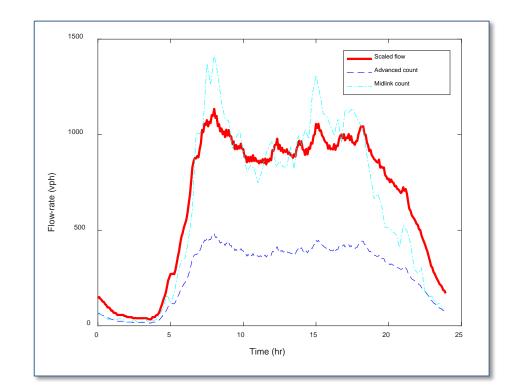
## Data Adjustments – Flow Rescaling

#### 

 Consistently low flows due to incomplete detector coverage

### Solution:

 Rescale observed approach flows using historical mid-link counts

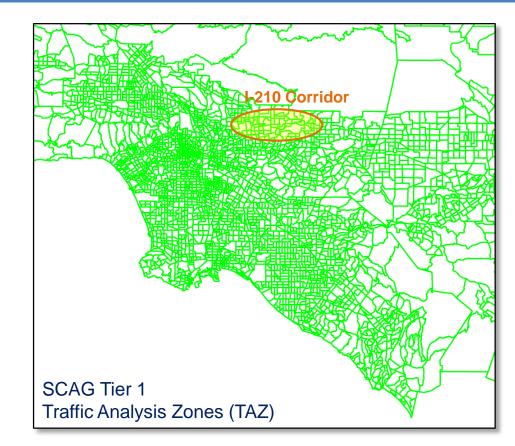






# What is Demand Modeling?

- Development of a table defining trips that people make within a network
  - Between specific zones
  - By time of day
  - By mode
  - By purpose
- Often related to demand model maintained by regional planning agencies





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# Demand Modeling Approaches

### Approach 1 - Specification of approach traffic flows and turn percentages at intersections

- Vehicles move through the network without a clear destination
- At each intersection, vehicles determine whether they go straight, turn right or turn left based on probabilities tied to observed data

### Approach 2 - Definition of origin-destination flow matrices

- Vehicles travel across a network based on their defined origin and destination
- Vehicles typically assigned to the route, or routes, having the lowest cost



# Demand Modeling Approach

- Similar to other leading commercial models, Aimsun uses origindestination matrices to model traffic demand
  - Provides greater flexibility in modeling routing applications

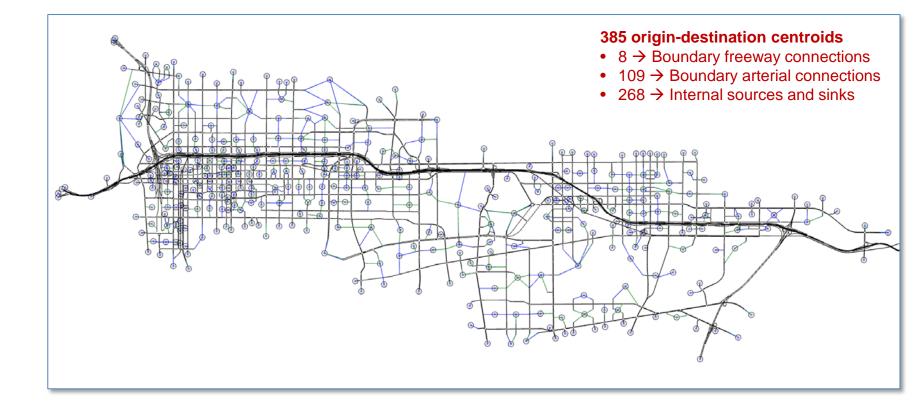
### Key modeling decisions

- Origin and destination nodes to include
  - Nodes at network boundaries representing incoming and outgoing traffic
  - Nodes representing traffic sources and sinks within the network
- Need to keep the number of nodes to a practical minimum
  - Simplifies data processing



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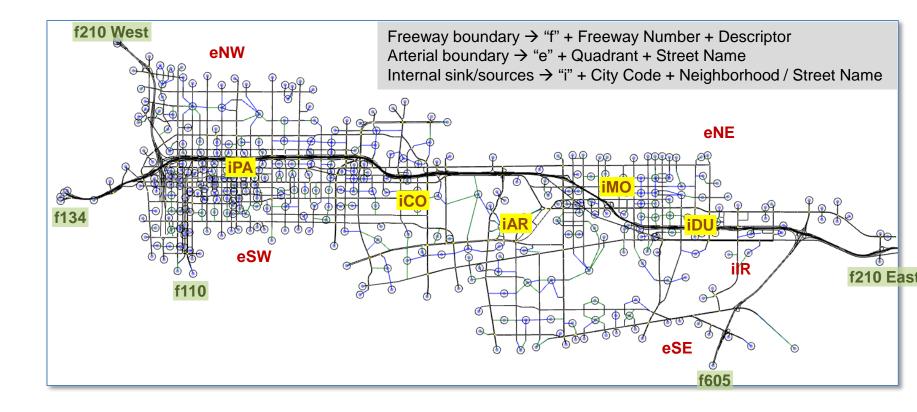
### Resulting origin-destination modeling





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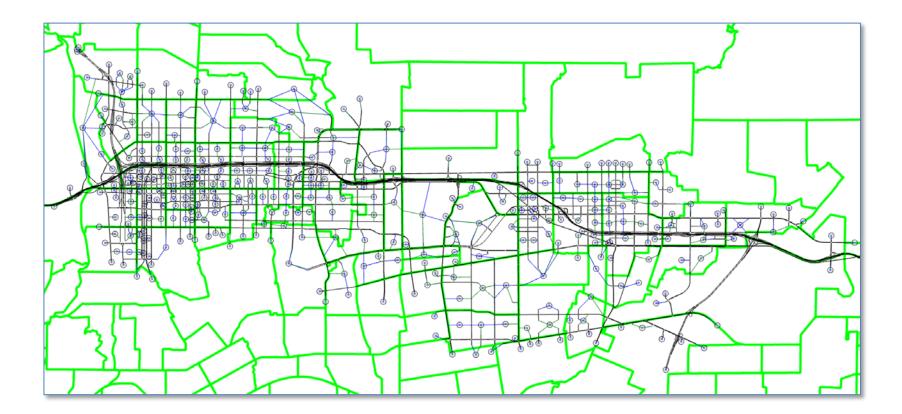
### Labeling of centroids for I-210 Corridor





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Correspondence between centroids and traffic analysis zones





### **Best practice:**

- Centroids are grouped by their corresponding geographical Traffic Analysis Zones (TAZ)
  - Simplify conversion of data obtained from regional travel demand models
- Specific centroids for elements that may be the focus of specific analyses
  - Parking facilities
  - Events centers











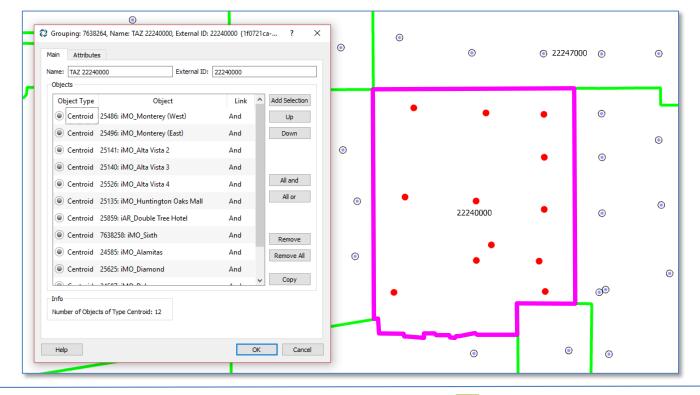




### **Demand Data Disaggregation**

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Except for special cases, demand for each TAZ is split equally among centroids in the TAZ











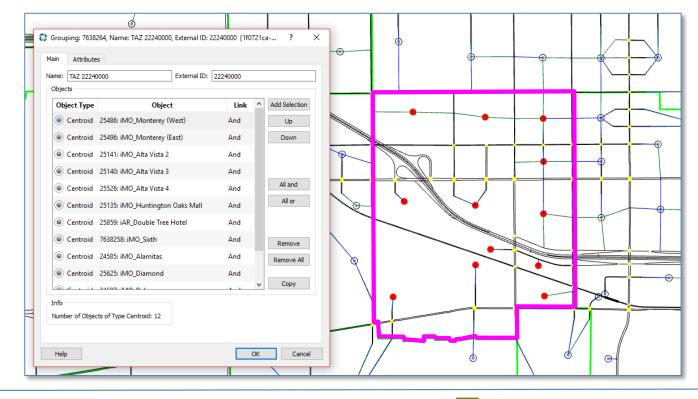




## **Demand Data Disaggregation**

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- Centroid may be connected to multiple places in the network
- **Flow allocation from centroid to network entrance is case-by-case**















## **Demand Data Sources**

### OD trip matrices from regional travel demand model

SCAG's 2012 TransCAD

### Traffic studies

■ Flows and turning counts from ~21 studies

### Traffic flow data from traffic monitoring systems

- Volume data from PeMS
- Volume data from mid-block / advanced / stop line traffic detectors

### Emerging data sources

Tracking data from probe vehicles / cell towers



# Estimation of O-D Trip Patterns

- Leading simulation models generally provide functions to estimate O-D trip patterns from observed traffic counts
  - Mathematical problem with multiple possible solutions
  - Solution search made more difficult by inconsistent data
  - Requires a lot of judgement calls

### Best to start with a seed matrix

- Aimsun developers suggest that using O-D matrix from regional travel demand model is usually the best starting point
- Limited research done to date on how to leverage emerging data sources:
  - Probe vehicle data
  - Cellular phone data records



# How Many Matrices to Develop?

- Which day(s) to model?
  - Average weekday
  - Individual weekdays
  - Average Saturday
  - Average Sunday
  - Hard Holidays
  - Soft Holidays



### What period of day to model?

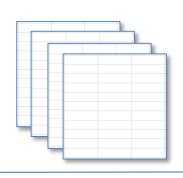
- AM peak period
- PM peak period
- Midday

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Evening/night

### Which types of vehicles

- Single-occupancy passenger cars
- High-occupancy passenger cars
- Medium-duty truck
- Heavy-duty trucks





# **Demand Elements and Examples**

- Day types
  - Weekday
  - Weekend
- Vehicle types
  - Cars
  - HOVs
  - Trucks

### Trip categories

- General
- Eastbound
- Westbound

### Time Periods

- AM
- PM

### Profiles

 Time slicing appropriate for different trip categories

### Scale factors

 Fine tuning adjustments for day subtype, or for incidents













# Data from SCAG Regional Demand Model

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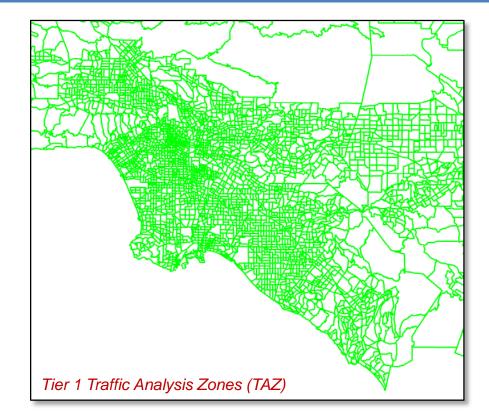
 Average weekday O-D trip matrices at TAZ level

#### Time periods

AM Peak:	6:00 AM - 9:00 AM
Midday:	9:00 AM - 3:00 PM
PM Peak:	3:00 PM - 7:00 PM
Evening:	7:00 PM - 9:00 PM
Night:	9:00 PM - 6:00 AM

#### Vehicle types

- HOV non-user 1 rider (drive alone)
- HOV non-user 2 riders
- HOV non-user 3 riders
- HOV user 2 riders
- HOV user 3 riders
- Light truck
- Medium truck
- Heavy truck











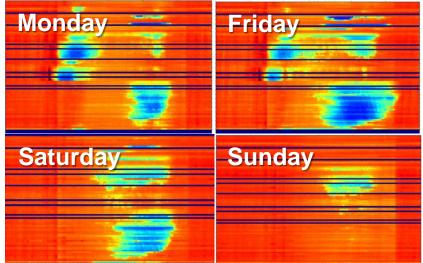


# Day Types

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### Number of day types determined by cluster analysis

- Initial analysis based on PeMS suggests the following minimum categorization
  - Ave Weekday (Mon-Thurs)
  - Friday
  - Saturday
  - Sunday
  - Hard holiday
  - Soft holiday



 Differences within a cluster can be represented by combinations of scale factors and profiles











# **Extended Vehicle Types**

- 4 vehicle types modeled
  - Car
  - HOV
  - Medium truck
  - Heavy truck

4 types X 3 categories 12 extended vehicle types

#### 3 trip categories to increase flexibility

- General trips
- Eastbound external-external trips
  - I-210 to I-210 E
  - I-210 to I-605 S
  - SR-134 to I-210 E
  - SR-134 to I-605 S
  - I-605 N to I-210 E
- Westbound external-external trips
  - I-210 to I-210 W
  - I-210 to SR-134
  - I-210 to I-605 S
  - I-605 N to I-210 W
  - I-605 N to SR-134





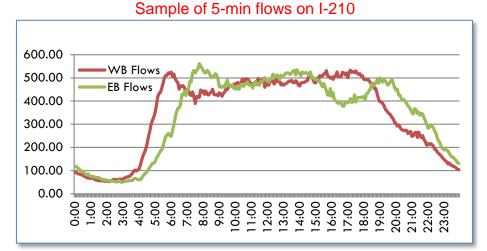
# **Time-slicing**

### Time granularity of 15 min

- 96 points in a 24-hour time profile
- Trips from each of the 5 SCAG time periods will be distributed based on diurnal profiles

### Three diurnal profiles

- One profile for each category:
  - General
  - Eastbound external-external,
  - Westbound external-external





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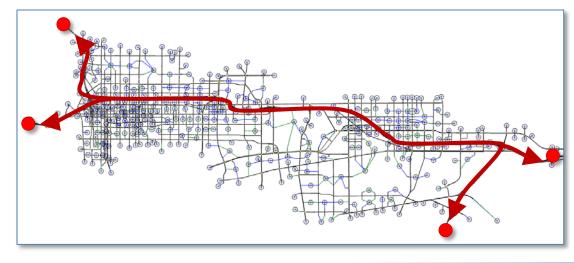






### **Demand prediction**

- Key destinations such as external-external flows at main gates are disaggregated by extended vehicle types
- Scaling of corridor traffic based on current day conditions
- During incidents, enable targeted scaling to modify expected changes to external-external flows







# **Route Selection Modeling**

### Route selection:

Process by which travelers determine which set of roadway links they will follow to reach their destination

### Factors affecting route selection

- Trip cost calculation
- Influence of trip cost on route selection
- □ I-210 corridor provide drivers with multiple possible paths → Increases sensitivity to route selection modeling





### By default, simulation models typically consider travel time as the only factor affecting route selection

- OK for small networks
- Can create issues in large networks, as routes with significant different length can have similar travel time due to differences in speed limits, traffic control effects
- Can push a significant proportion of travelers to choose longer route

### Next refinement is to add travel distance as a factor

- Tends to prevent vehicles from choosing "sightseeing" routes
- Requires some custom programming



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#### Example: Route selection based solely on travel time















#### Example: Route selection considering travel time and distance















### Further refinement: factoring traveler preferences

- For freeway and arterial trips with similar travel times → motorists tend to prefer using the freeway
- Motorists may be willing to travel extra distance to stay on a freeway
- HOV vehicles have extra incentive to use HOV lanes



#### Static traffic assignment trip costs

Arterials:Travel Time + 1.00 \* Distance + Ramp Meter PenaltyFreeways – Main lanes:Travel Time + 0.85 \* Distance + Ramp Meter PenaltyFreeway - HOV lanes:Travel Time + 0.80 \* Distance + Ramp Meter Penalty



# **Routing Data Sources**

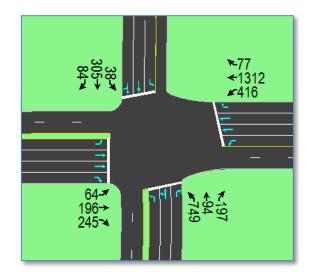
### Traditional sources

- Volume along roadway segments
- Turning counts at intersections
- Observations from traffic managers



No direct observations of paths taken

Paths followed by vehicles inferred from observed data

















# **Routing Data Sources**

#### Emerging sources

- Tracking data from equipped vehicles (probe vehicles)
- Tracking data from GPS-equipped smartphones
- Tracking data cellular phone network operators

#### Potential benefits

Direct observation of travel paths

#### Potential issues

- **GPS** location accuracy
- Distinguishing phones carried onboard vehicles and by pedestrians in congested urban areas
- Distinguishing multiple phones being tracked from same vehicle







# Routing Assumptions – Base Model

Initial distribution of route selection models within simulated traffic fleet

- 80% of passenger cars follow distribution of shortest paths produced by Dynamic User Equilibrium (DUE) assignment
  - Up to 3 paths considered between each O-D pair
- 20% of passenger cars able to recalculate paths en-route
  - C-Logit model
- Slight variations for medium and heavy trucks
- Percentages to be adjusted during AMS model calibration





### **Response Plan Modeling Elements**

#### 

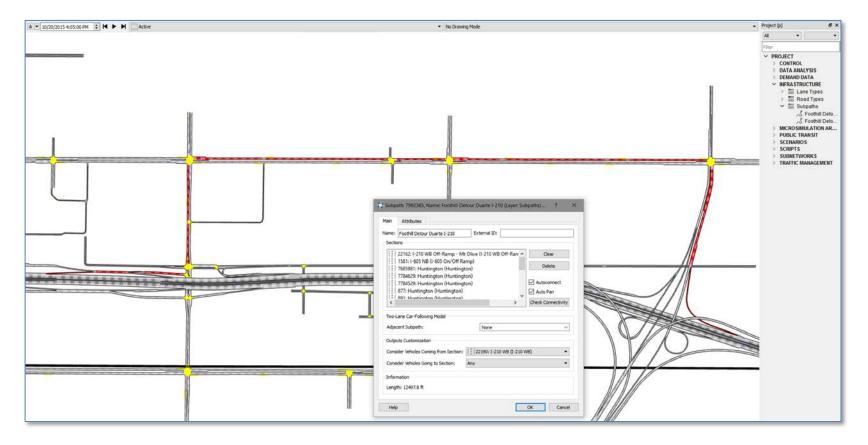
- Driver responses
- Response timing plans



## Incident Response – Modeling of Detours

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### Identification of all possible detours in Aimsun

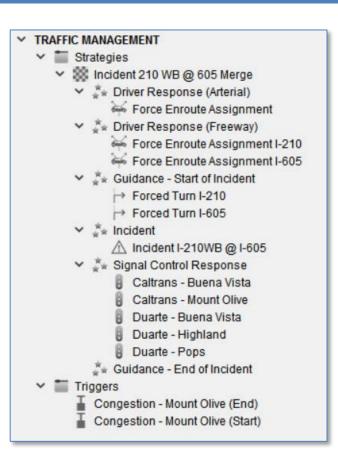




# Incident Response – Driver Behavior

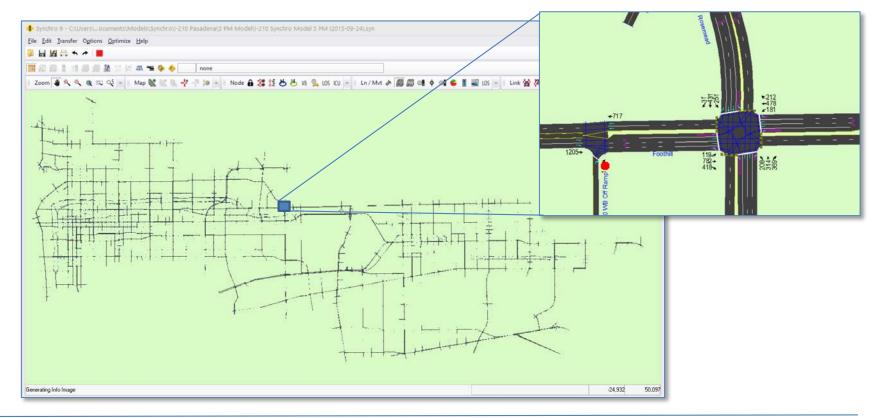
### Parameters to be defined for modeling incidents and response plans

- Locations where motorists receive guidance
- Detour route(s) followed
- % drivers accepting guidance
- % other drivers adjusting their route based on observed traffic conditions
- Event triggering driver responses
- Percentages to be determined based on literature review, Berkeley research, modeling experience, etc.



## Incident Response – Timing Plans

Preliminary timing plans to be developed using available Synchro model of corridor

















# **Running Large Scale Models**

#### Execution time is a key concern when running large simulation models

- Not an issue in conducting off-line analyses
- Critical issue: model is to be used in a real-time operational context

### Traditional solution

- Running the model on computers with high processing power
- Parallelizing the simulation process to allow multiple threads

### Emerging solution

 Moving execution to the cloud, where computing power could be scaled as needed

















# **AMS and Research Themes**

- Machine Learning for Calibration and Control
- Data Quality Algorithms
- Estimation Methods
- Data Fusion Methods
- Call Data Records (CDRs) for OD
- Probe Data for ODME, Speed and Density
- Route Choice
- Driver Response to Guidance or Incentives



# I-210 Corridor as a Center for Future Transportation Innovation

### The I-210 Corridor will be uniquely positioned

- Well studied, well instrumented, well understood, and well modeled
- Centralized archives of high quality data over diverse operating conditions
- Leveraging these assets, the I-210 will be an ideal corridor for future innovation and new technologies
  - Connected and automated vehicles
  - Transportation as a service
  - Coordination between public and private modes
  - Integration with smart cities and regions
  - New partnerships and new possibilities



# Partnerships and Cooperation

- To push forward the state-of-the-art and the state-of-practice we plan to share our:
  - Models
  - Data
  - Algorithms
- We are interested in partnering with:
  - Stakeholders
  - Academic institutions
  - Federal and state governments
  - Industry partners





### Questions for discussion?